

STONEFIELD

STORMWATER MANAGEMENT REPORT PRIMROSE SCHOOL FRANCHISING COMPANY

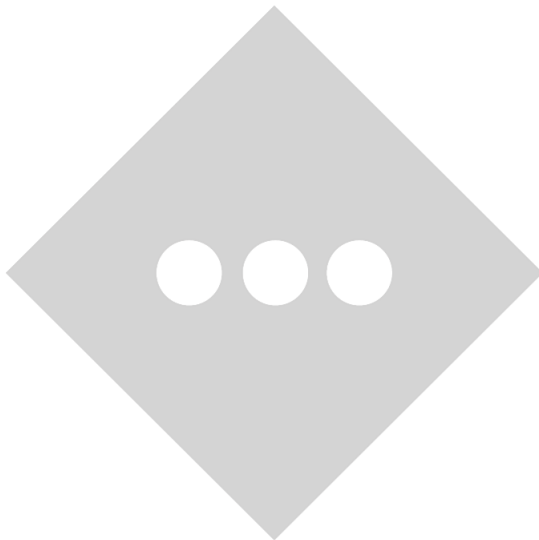
PROPOSED CHILDCARE FACILITY
PARCEL ID: 28-113
885 MAIN STREET
TOWN OF READING
MIDDLESEX COUNTY, MASSACHUSETTS

PREPARED FOR:
PRIMROSE SCHOOLS FRANCHISING COMPANY
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1.0 PROJECT DESCRIPTION

Primrose School Franchising Company is proposing to develop Parcel 28-113, commonly known as 885 Main Street, Reading, MA, (herein referred to as the “project site”) to accommodate the construction of a 14,058 square foot Childcare Facility (7,029 SF Floor Plate). Additional improvements include children’s playground areas with associated play equipment, lighting, landscaping, off-street parking facilities, utility connections, and stormwater infrastructure.

The property is located within the Single Family 15 (S-15) zoning district in the Town of Reading. The proposed development fronts Main Street (Route 28) and is surrounded by single family residential lots. The site will be accessed via one (1) full movement driveway from Main Street (MA Route 28). Refer to **APPENDIX A** for project maps of the subject site.

The project site is 84,280 SF (1.94 acres), the extent of land disturbance is 58,951 SF (1.35 acres), and 19,481 SF (0.45 acres) of impervious surface will be added to the project site. The overall drainage area was modeled as 84,280 SF (1.94 acres).

This Report has been prepared to analyze the potential stormwater runoff impacts of the proposed project site and outline proposed measures to conform to the stormwater management regulations set forth by the Town of Reading and the Massachusetts Department of Environmental Protection.

2.0 EXISTING CONDITIONS

EXISTING SITE DEVELOPMENT

The project site fronts Main Street (MA Route 28) to the West. Under existing conditions, the project site is developed with an approximately 3,070 ± SF house with associated accessory structures, parking facilities, pool and utility connections. The site is accessed via one (1) full movement driveway off Main Street. There is an existing bordering vegetated wetland on the northeast corner of the project site that captures all runoff within the existing developed area. The entirety of the existing structures, associated parking area, and utility connections will be removed to accommodate the proposed development. An Aerial Map depicting the existing site conditions can be found in **APPENDIX A**.

EXISTING TOPOGRAPHY

The high point of the project site is 125.0’ along the southern property line near Francis Drive. Runoff will sheet flow north from the high point near Francis drive, across site, and discharge to the onsite wetlands. Another high point of 112.5’ exists at the western edge of the project site along Main Street, runoff from this point, similarly, will

sheet flow across site and discharge to the wetlands. No runoff from the project site enters the State Highway Layout. Grades onsite generally range from 2-7% within the previously developed area and increases to 30-35% as it approaches the onsite wetlands to the Northeast.

PROJECT SITE SOILS

Soil mapping was obtained from the National Resource Conservation Service (NRCS) for the project site and immediate area. Generally, the project site is underlain with one major soil group: Sandy Loam (SL). Overall, the soils are well drained and runoff flows overland northeast to the on-site wetlands. The table below provides a summary of soils for the project site:

TABLE I: NRCS SOIL MAPPING RESULTS

Soil Unit Code	Soil Description	Approximate Project Coverage	Drainage Class	Hydrologic Soil Group
73B	Whitman Fine Sandy Loam	61.3%	Very Poorly Drained	D
631C	Charlton-Urban Land-Hollis Complex	28.5%	Well Drained	A
655	Udorthents	5.9%	NS	D*
305C	Paxton Fine Sandy Loam	3.8%	Well Drained	C
629C	Canton-Charlton-Urban Land Complex	0.5%	Well Drained	A

*629C does not have a pre-determined hydraulic soil group. As such, these soils are analyzed as HSG D for a conservative analysis.

Additional information regarding the NRCS soil mapping can be found in **APPENDIX B**.

A “Preliminary Stormwater Management Area Evaluation” was performed by Whitestone Associates (report dated March 14, 2025), which included 4 Test Pits, 3 of which included infiltration testing. The soil conditions encountered within the subsurface tests conducted by Whitestone consisted of the following generalized strata in order of increasing depth. *Records of Subsurface Exploration* are provided in Appendix A.

Surface Cover Materials: The test pits encountered 3.0 inches to 5.0 inches of topsoil at the ground surface. In test pits TP-2 and TP-4, the topsoil was underlain by 10 inches and 19 inches, respectively, of subsoil with roots.

Existing Fill: Beneath the surface cover materials, test pits TP-1 and TP-3 encountered existing fill, consisting of gray to brown, silty sand with gravel to poorly graded sand with silt and gravel, cobbles, boulders. The existing fill extended to depths of 6.5 fbg and 3.5 fbg in test pits TP-1 and TP-3, respectively. In test pit TP-1, the existing fill was underlain by 6.0 inches of former topsoil and 6.0 inches of former subsoil with roots. In test pit TP-3, the existing fill was underlain by 6.0 inches of former topsoil.

Glacial Till: Beneath the surface cover materials or former topsoil/subsoil, the test pits encountered glacial till, consisting of brown, silty sand with gravel (USCS: SM) or silty gravel with sand (USCS: GM), cobbles, boulders. The test pits were terminated in the glacial till at a depth of 10 fbs.

Groundwater: Groundwater was encountered in only test pit TP-2 during the exploration at a depth of 10 fbs. As noted above, no indications of ESGWH were observed in the test pits. Groundwater levels should be expected to fluctuate seasonally and following periods of precipitation.

WATERSHED / RECEIVING WATERS – TMDL DESIGNATION

Under existing conditions, the site drains to the onsite bordering vegetated wetlands to the northeast of the project site that ultimately discharges to Saugus River (State Waterbody ID: MA93-94). The watershed for the development is part of the Headwaters Saugus River Watershed (State Watershed ID Designation: 010900010401) as defined by the United States Environmental Protection Agency for Community Waterway Mapping. Per the Massachusetts Year 2022 Integrated List of Waters prepared by the Massachusetts Department of Environmental Protection, Saugus River is identified as an impaired water for algae, bacteria and other microbes, degraded habitat, low oxygen, murky water, and nitrogen and/or phosphorus.

EXISTING ENVIRONMENTAL INVENTORY

Based on the effective FEMA flood insurance rate mapping (FEMA Map #25017C31 IE issued June 4th, 2010), the entirety of the site lies within flood zone x, an area with minimal flood hazard. The FEMA Map can be found in **APPENDIX A** of this Report.

There are state (MassDEP) regulated freshwater wetlands within the project site that are subject to the Wetlands Protection Act Regulations (310 CMR). As there are regulated wetlands within the project site, the limits of the areas and associated Buffer Zones are shown on the Site Plans prepared by Stonefield in conjunction with this Report. Per the records of natural communities maintained in the National Heritage & Endangered Species Program (NHESP) database, there are no records of endangered or threatened species sightings or suitable habitats located within the vicinity of the proposed improvements.

3.0 PROPOSED CONDITIONS

PROPOSED SITE DEVELOPMENT

The proposed development will consist of a 14,058 square foot child daycare facility. Additional improvements include an off-street parking facility, lighting, landscaping, child play area, utility services and stormwater management infrastructure. The site will be accessed via one (1) full movement driveway off of Main Street. Refer to **APPENDIX A** for a half-size Overall Site Plan depicting the proposed project improvements.

PROPOSED TOPOGRAPHY

Project site topography and drainage patterns will generally remain similar to existing conditions; however, due to the need for more commercially friendly, ADA compliant grades, the previously developed area has been widened and flattened. A combination of extended curbing and retaining/landscape walls will be implemented through the project to make up for the change in grades.

ANTICIPATED ENVIRONMENTAL INVENTORY IMPACTS

The proposed development will not disturb land within the 25’ wetland buffer. The Township will remain apprised of the MassDEP permitting status as the project moves forward.

4.0 STORMWATER MANAGEMENT METHODOLOGY & PARAMETERS

HYDROLOGIC METHODOLOGY

The analysis program “HydroCAD” Version 10.0 by HydroCAD Software Solutions was utilized to calculate and plot the runoff hydrographs. The program incorporates the time of concentration, C values, rainfall data, and project drainage areas to calculate the runoff characteristics. The existing and proposed drainage areas have been analyzed utilizing Intensity-Duration-Frequency data obtained from NOAA for the project area; specifics of the rainfall distribution can be found in **APPENDIX C**. Additional key variables utilized in the analysis include:

TABLE 2: HYDROCAD DESIGN VARIABLES

Variable	Input	Variable	Input
Runoff Calculation Method	SCS TR-20	NRCS Rainfall Frequency Data Set	Middlesex
Pervious/Impervious CN Calculations	Separate	Storm Intervals (Year Events)	2, 10, 25, 100
Stage-Storage Relationship	Dynamic	Storm Duration	24 Hours
Minimum time of concentration	6 minutes	Storm Curve	NOAA D

Additional information regarding the hydrologic calculations can be found in **APPENDIX C**.

5.0 STORMWATER ANALYSIS

EXISTING DRAINAGE AREAS

Under current conditions, the project site is comprised of one (1) drainage area discharging to one (1) Point of Interest (POI-I). The ultimate POI analyzed for the development is the existing bordering vegetated wetlands located in the northeast corner of the project site. POI-I, comprised of drainage area EX-I, receives runoff via sheet flow from the entirety of site. See below for a short summary of the drainage area:

TABLE 3: SUMMARY OF EXISTING DRAINAGE AREA

Drainage Area	Description	Area Extents	Impervious Area	Time of Concentration
EX-1 (POI-1)	Existing Runoff to Wetlands	84,280 SF	11,547 SF	11.2 Minutes

Existing drainage areas were delineated based on field surveying data. Hydrologic calculations and parameters for each drainage area can be found in **APPENDIX C**; specific drainage area delineations and land cover can be found in **APPENDIX E**.

PROPOSED DRAINAGE AREAS

Under proposed conditions, the general drainage patterns and ultimate point of interest will be maintained. The intent behind the proposed delineations is to reduce the amount of direct runoff to the onsite bordering vegetated wetlands. The diverted land from drainage area P-1B is sent to various stormwater management features to meet the Massachusetts Department of Environmental Protection Stormwater Management Standards as outlined in the next Report section. See below for a short summary of each area:

TABLE 5: SUMMARY OF PROPOSED DRAINAGE AREAS

Drainage Area	Description	Area Extents	Impervious Area	Time of Concentration
P-1A	Proposed Drainage Direct to POI-1	54,756 SF	2,095 SF	6.6 Minutes
P-1B	Proposed Drainage to B-1	29,525 SF	28,933 SF	6.0 Minutes*
POI (P-1)	Ultimate Point of Interest: Onsite Bordering Vegetated Wetlands	84,280 SF	26,805 SF	N/A

***The minimum time of concentration was utilized due to the high level of impervious coverage / land disturbance and proximity to existing and proposed stormwater pipe conveyance systems**

All proposed drainage areas were delineated based on the proposed grading design overlain on field survey data. Hydrologic calculations and parameters for each drainage area can be found in **APPENDIX C**; specific drainage area delineations and land cover can be found in **APPENDIX E**.

STORMWATER MANAGEMENT DESIGN PARAMETERS

The extent of development proposes to disturb over one (1) acre of the existing site; as such, it is subject to all Stormwater Standards as defined in the Town Ordinances and the Massachusetts Stormwater Handbook Volume I. See below for a summary of each design parameter and compliance requirements:

TABLE 6: STORMWATER DESIGN STANDARDS SUMMARY

Design Parameter	Design Target for Compliance
Standard 1: <i>Stormwater Discharge</i>	Demonstrate that no new stormwater conveyances will discharge untreated stormwater directly to or cause erosion in wetlands or waters.
Standard 2: <i>Stormwater Quantity</i>	Demonstrate through hydrologic and hydraulic analysis that for stormwater leaving the site, post-construction runoff hydrographs for the 2-, 10-, and 100-year storm events do not exceed, at any point in time, the pre-construction runoff hydrographs for the same storm events.
Standard 3: <i>Groundwater Recharge</i>	Demonstrate through hydrologic and hydraulic analysis that the site and its stormwater management measure shall approximate average annual pre-construction groundwater recharge volume for the site.
Standard 4: <i>Stormwater Quality</i>	Stormwater management measures shall be designed to reduce the post-construction load of total suspended solids (TSS) in stormwater runoff generated from the water quality design storm by 80 percent of the anticipated load from existing and proposed impervious coverage onsite. In accordance with Town Stormwater Regulations section 4.1.3.4., all new development sites shall be designed to meet an average annual pollutant removal equivalent to 90% of the average annual load of Total Suspended Solids (TSS) related to the total post-construction impervious area on the site And 60% of the average annual load of Total Phosphorus (TP) related to the total post-construction impervious surface area on the site.
Standard 5: <i>High Pollutant Loads</i>	Demonstrate that the discharge of stormwater runoff from land uses with higher potential pollutant loads will be eliminated or reduced through complete protection from potential runoff or use of a specific structural BMP.
Standard 6: <i>Critical Areas</i>	The project does not lie within Zone II Areas, Interim Wellhead Protection Areas, Outstanding Resource Waters, Special Resources, Zone I, or Zone A and therefore is exempt from meeting Standard 6 as it is not applicable to the development.

STANDARD 1 – STORMWATER DISCHARGE

The proposed stormwater conveyance system for POI-I discharges directly to the onsite bordering vegetated wetlands and is therefore subject to water treatment standards. The level of treatment is outlined under the Standard 4 section in this Report as the development impacts a critical area.

STANDARD 2 – STORMWATER QUANTITY

A series of SC-800 StormTech chambers in conjunction with an outlet control structure are used to attenuate peak stormwater runoff rates to the mandated regulatory levels. The tables below summarize the various drainage areas in relation to flow rates and runoff volume during regulatory storm events:

TABLE 7: SUMMARY OF EXISTING DRAINAGE AREA FLOW RATES

Drainage Area	2-Year Flow Rate	10-Year Flow Rate	25-Year Flow Rate	100-Year Flow Rate
POI (E-I)	1.49 CFS	3.34 CFS	4.98 CFS	8.59 CFS

TABLE 8: SUMMARY OF PROPOSED DRAINAGE AREA FLOW RATES

Drainage Area	2-Year Flow Rate	10-Year Flow Rate	25-Year Flow Rate	100-Year Flow Rate
P-1A (Undetained to POI)	1.21 CFS	2.74 CFS	4.07 CFS	6.94 CFS
P-1B (To Infiltration System)	1.85 CFS	2.82 CFS	3.57 CFS	5.11 CFS
POI (P-1)	1.21 CFS	2.74 CFS	4.07 CFS	8.33 CFS

Under post-development conditions the runoff flow rates and volumes are reduced to the bordering vegetated wetlands, the ultimate point of interest (POI-1). Runoff is diverted from the majority of the developed area (P-1B) to the on-site stormwater management system for runoff attenuation and water quality treatment. The table below outlines the regulatory compliance parameters for runoff quantity on the project site:

TABLE 9: STORMWATER RUNOFF QUANTITY COMPLIANCE SUMMARY (POI-1)

Rainfall Event	Existing Flow Rate	Proposed Flow Rate	Proposed % Reduction
2-Year Storm	1.49 CFS	1.21 CFS	18.79%
10-Year Storm	3.34 CFS	2.74 CFS	17.96%
25-Year Storm	4.98 CFS	4.07 CFS	18.27%
100-Year Storm	8.59 CFS	8.33 CFS	3.03%

The proposed SC-800 StormTech chambers in conjunction with an outlet control structure provide sufficient flow rate attenuation to ensure that no adverse impacts are anticipated downstream of the project site. Detailed hydrologic calculations for each drainage area can be found in **APPENDIX C**.

STANDARD 3 – GROUNDWATER RECHARGE

Groundwater recharge is required as the infiltration rates of the soils were found to be greater than 0.17 in/hour and there are no contaminated soils on or within the vicinity of the site. Groundwater recharge is met through the implementation of the aforementioned subsurface infiltration system which provides a total recharge volume of

2,290 CF. There is a separation of ± 4.0 feet between the bottom of the infiltration structure and the seasonal high groundwater table, therefore a groundwater mounding analysis was not required.

The required recharge volume was calculated by multiplying the total impervious area, 31,028 SF, by 0.60 inches due to the hydrologic rating of A which yields a required recharge volume of 1,551.4 CF. The proposed BMP exceeds this requirement by 1874.6 CF and therefore meets Standard 3 requirements. The dynamic method was utilized and can be found in **APPENDIX C**.

STANDARD 4 – STORMWATER QUALITY CONTROL

For all developments, a removal of 80% of the average annual post-construction load of Total Suspended Solids (TSS) is required.

Compliance with stormwater runoff quality requirements will be accomplished through street sweeping (9% TSS removal rate), deep sump and hooded catch basins (25% TSS removal rate), ADS isolator Row (25% TSS removal rate), and Infiltration Basin (80% TSS removal rate), which when utilized in series provide a combined 90% TSS removal rate. The table below summarizes the required and proposed weighted TSS removal rates:

The proposed treatment design will exceed the regulatory requirements for stormwater runoff quality and ensure that runoff discharged into the unnamed tributary will not have any adverse effects on downstream waterways and environs. The MassDEP TSS removal spreadsheet can be found in **APPENDIX C**.

Per the Town of Reading Stormwater Regulations, all new development sites shall be designed to meet an average annual pollutant removal equivalent to 90% of the average annual load of Total Suspended Solids (TSS) related to the total post-construction impervious area on the site AND 60% of the average annual load of Total Phosphorus (TP) related to the total post-construction impervious surface area on the site.

Compliance with the Towns stormwater runoff quality requirements will be demonstrated through method outlined in section 4.1.4.4.2 of the Town of Reading Stormwater Regulations: “Retaining the volume of runoff equivalent to, or greater than, one (1.0) inch multiplied by the total post-construction impervious surface area on the new development site”.

The required volume to be retained onsite was calculated by multiplying the total area of impervious surface, 31,028 SF, by one (1) inch which yields a volume of 2,586 CF. The proposed BMP exceeds this requirement by 840 CF and therefore meets the local pollutant removal requirements.

STANDARD 5 – HIGH POLLUTANT LOADS

The proposed use for the development is a child day care facility which is not considered a Land Use with Higher Potential Pollutant Loads (LUHPPL) by the MassDEP and therefore is exempt from Standard 5 requirements.

STANDARD 6 – CRITICAL AREAS

The proposed redevelopment area does not lie in or discharge to a Zone II Interim Wellhead Protection Area, Outstanding Resources Waters, Special Resource Waters or other critical area as defined by the Massachusetts Stormwater Handbook Volume I, and therefore is exempt from Standard 6 requirements.

STANDARD 7 – REDEVELOPMENT PROJECT

Since the site has a net increase of impervious cover by 19,481 SF (0.45 AC), the site is not determined to be a redevelopment project and must comply with all Standards as defined in the Massachusetts Department of Environmental Protection Stormwater Management Standards.

STANDARD 8 – EROSION, SEDIMENTATION, AND POLLUTION PREVENTION PLAN

A Soil Erosion & Sediment Control Plan has been prepared in accordance with the latest edition of Volume 2 of the Massachusetts Stormwater Handbook and the Erosion and Sedimentation Control Guidelines. This plan can be found within the Land Development Plans prepared by Stonefield Engineering & Design in conjunction with this Report. Proposed temporary measures during construction include but are not limited to silt fencing, stabilized construction entrance, inlet filters, silt sock, street sweeping, and temporary seeding for soil stabilization. No land disturbance will occur until certification and permits have been obtained. Details for all proposed control measures have also been provided.

STANDARD 9 – STORMWATER FACILITY OPERATIONS AND MAINTENANCE

A Stormwater Operations & Maintenance Manual has been included in this Pollution Prevention Plan. Any necessary easements or covenants associated with the stormwater improvements will be recorded prior to the start of construction.

STANDARD 10 – ILLICIT DISCHARGES

The proposed stormwater management system discharges are entirely comprised of stormwater. Firefighting, water line flushing, landscape irrigation, uncontaminated groundwater, potable water sources, foundation drains, air conditioning condensation, footing drains, and water for street washing are prohibited to discharge onsite and will therefore not result in an illicit discharge.

6.0 EROSION, SEDIMENTATION, AND POLLUTION PREVENTION

TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES

Under proposed conditions, erosion and sediment controls will be utilized to limit the potential effects due to construction of the proposed development. Refer to the Soil Erosion and Sediment Control Plans in **APPENDIX A** of this report. The following includes the temporary sediment controls proposed for this project:

Construction Entrance – To provide a stable entrance and exit from a construction site and keep mud and sediment off public roads, a temporary stone-stabilized pad located at points of vehicular ingress and egress on a construction site. If the action of the vehicle traveling over the gravel pad is not sufficient to remove the majority of the mud, then the tires must be washed before the vehicle enters a public road. If washing is used, provisions must be made to intercept the wash water and trap sediment before it is carried off-site.

Dust Control – To reduce surface and air movement of dust from exposed soil surfaces during land disturbing, demolition, and construction activities, preventative measures must be taken. Sprinkling or other approved methods must be used to reduce dust generated on the site. Dust control shall be provided by the general contractor to a degree acceptable to the owner/operator, and in compliance with the applicable local and state dust control requirements.

Inlet Protection – A sediment filter or an excavated impounding area around a storm drain, drop inlet, or curb inlet must be used to prevent sediment from entering storm drainage systems prior to permanent stabilization of the disturbed area. During construction, the inlet protection measures shall be replaced as needed to ensure proper function of the structure.

Preserving Natural Vegetation – Natural vegetation should be preserved whenever possible, but especially on steep slopes, near perennial and intermittent watercourses or swales, and on building sites in wooded areas. Clearly flag or mark areas around trees that are to be saved. It is preferable to keep ground disturbance away from the trees at least as far out as the dripline. If possible, place a barrier/fencing around the trees. Inspect flagged areas regularly to make sure flagging has not been removed. If tree roots have been exposed or injured, re-cover and/or seal them.

Sediment Fence – A temporary sediment barrier consisting of a filter fabric stretched across and attached to supporting posts and entrenched must be established along the perimeter of areas to be disturbed before initiation of and during construction. The sediment fence is constructed of stakes and synthetic filter fabric with a rigid wire fence backing where necessary for support. Sediment fence can be purchased with pockets pre-sewn to accept use of steel fence posts. Silt fences should be inspected immediately after each rainfall and at least daily during prolonged

rainfall. Repair as necessary. If the fabric tears, decomposes, or in any way becomes ineffective, replace it immediately. Replace burlap used in sediment fences after no more than 60 days.

Compost Filter Sock – A temporary tubular mesh sleeve that contains compost of a well-shredded organic material for a linear treatment that provides stormwater pollutant removal through filtration of pollutants from overland flow. The compost filter sock is placed at the bottom of the silt fence and should be repaired as necessary. Filter socks shall be inspected immediately after each rainfall and at least daily during prolonged rainfall as well as at least once weekly. If the fabric tears, decomposes, or in any way becomes ineffective, replace it immediately. Filter socks shall be replaced after 6 months. Upon completion of temporary control, the sock may be cut open and the mulch spread as a soil supplement.

Temporary Soil Stockpile – Locate the topsoil stockpile so that it does not interfere with work on the site. Side slopes of the stockpile should not exceed 2:1. Surround all topsoil stockpiles with an interceptor dike with gravel outlet and silt fence. Either seed or cover stockpiles with clear plastic or other mulching materials within 7 days of the formation of the stockpile. Topsoil should not be placed while in a frozen or muddy condition, when the subgrade is excessively wet, or when conditions exist that may otherwise be detrimental to proper grading or proposed sodding or seeding. Do not place topsoil on slopes steeper than 2:1. Maintain protective cover on stockpiles until needed.

PERMANENT EROSION AND SEDIMENT CONTROL MEASURES

Permanent Seeding – Permanent seeding of grass and planting trees and shrubs shall be established on any graded or cleared area where long-lived plant cover is needed to stabilize the soil in accordance with the accompanying plans. Areas which will not be brought to final grade for a year or more shall also be seeded permanently. Inspect seeded areas for failure and make necessary repairs and reseed immediately. Conduct or follow-up survey after one year and replace failed plants where necessary.

Riprap – A permanent, erosion-resistant ground cover of large, loose, angular stone must be installed in accordance with the accompanying plans to protect slopes, streambanks, channels, or areas subject to erosion by wave action. Riprap should be checked at least annually and after every major storm for displaced stones, slumping, and erosion at edges, especially downstream or downslope. If the riprap has been damaged, it should be repaired immediately before further damage can take place.

CONSTRUCTION PHASING PLAN AND SEQUENCE OF OPERATIONS

The Soil Erosion & Sediment Control Plans have been phased in order to effectively control erosion and sedimentation and minimize impacts due to seasonal changes. Please refer to **APPENDIX A** for half size Soil Erosion & Sediment Control Plans for detailed construction sequencing.

FINAL SITE STABILIZATION

Recommended practices for final surface stabilization include surface roughening, terrace, topsoiling, permanent seeding, sodding, trees and shrub planting, mulching, and riprap. The stabilization measures shall be in conformance with the *Massachusetts Erosion and Sediment Control Guidelines for Urban and Suburban Areas*, latest edition.

7.0 CONCLUSIONS

As demonstrated in this Report, the increase in runoff flow rate and volume generated by the proposed development will be satisfactorily mitigated by the introduction of an on-site stormwater conveyance system, a subsurface infiltration system, and an outlet control structure and on-site stormwater conveyance system. Runoff water quality will be impacted by the increase in impervious surfaces and a series of street sweeping, deep sump hooded catch basins, ADS isolator row and a subsurface infiltration system will provide treatment to remove total suspended solids to a satisfactory regulatory level. Groundwater recharge also will be impacted due to the loss of pervious surfaces and a subsurface infiltration system will provide groundwater recharge equal to or greater than recharge under existing conditions.

The proposed project complies with all applicable stormwater management regulations and standards. As such, the project is not anticipated to have any adverse drainage impacts on neighboring properties, downstream watercourses, or adjoining conveyance systems.

8.0 REFERENCES

1. Massachusetts Stormwater Handbook and Stormwater Standards, last amended January 2, 2008
<https://www.mass.gov/guides/massachusetts-stormwater-handbook-and-stormwater-standards>
2. Massachusetts Complete Erosion and Sedimentation Control Guidelines for Urban and Suburban Areas: A Guide for Planners, Designers, and Municipal Officials, last amended May 2003
<https://www.mass.gov/doc/complete-erosion-and-sedimentation-control-guidelines-a-guide-for-planners-designers-and/download>
3. Town of Reading Zoning Bylaw, last amended April, 2022
<https://www.readingma.gov/DocumentCenter/View/2242/Zoning-Bylaw-PDF>
4. Town of Reading Stormwater Management and Erosion Control Regulations, last amended December 6, 2021
<https://www.readingma.gov/DocumentCenter/View/2280/Stormwater-Regulations-PDF>

APPENDIX A PROJECT FIGURES

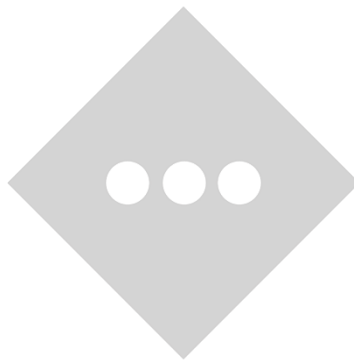
INVENTORY

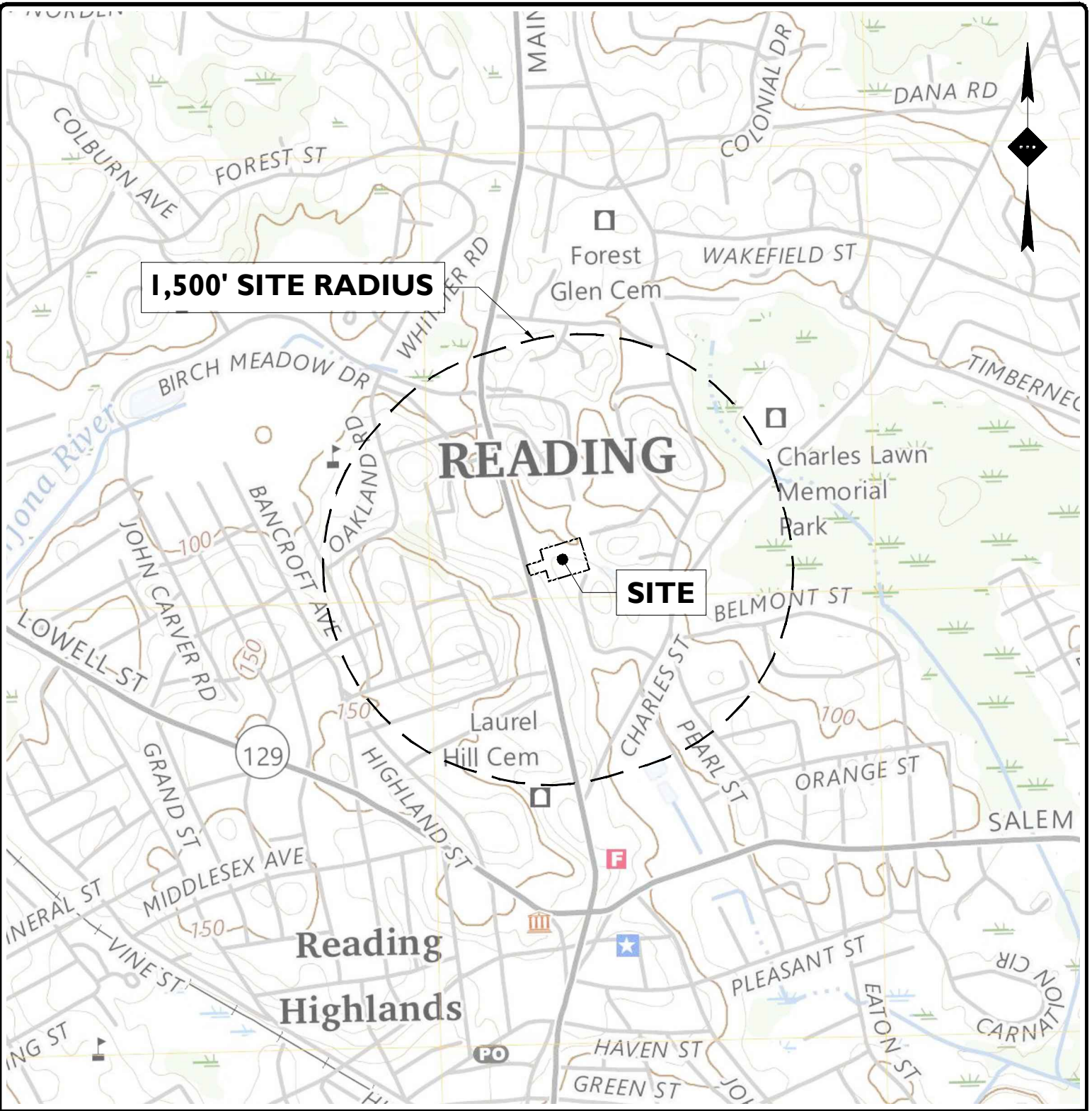
FIGURE 1: USGS LOCATION MAP

FIGURE 2: AERIAL MAP

FIGURE 3: TAX & ZONING MAP

FIGURE 4: FEMA MAP





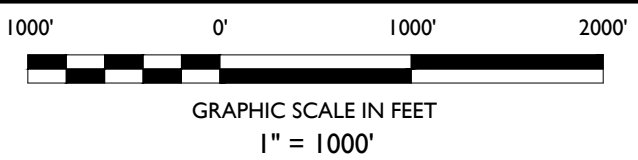
1,500' SITE RADIUS

SITE

READING

**Reading
Highlands**

USGS QUAD MAP



SOURCE: USGS READING QUADRANGLE MASSACHUSETTS 7.5-MINUTE SERIES

**PRIMROSE SCHOOLS FRANCHISING COMPANY
PROPOSED CHILD DAY CARE FACILITY**



PARCEL ID: 28-113
885 MAIN STREET, TOWN OF READING
MIDDLESEX COUNTY, MASSACHUSETTS

DRAWN BY:	SCL
CHECKED BY:	JHK
DATE:	02/27/2025
SCALE:	1" = 1000'
PROJECT ID:	BOS-240115



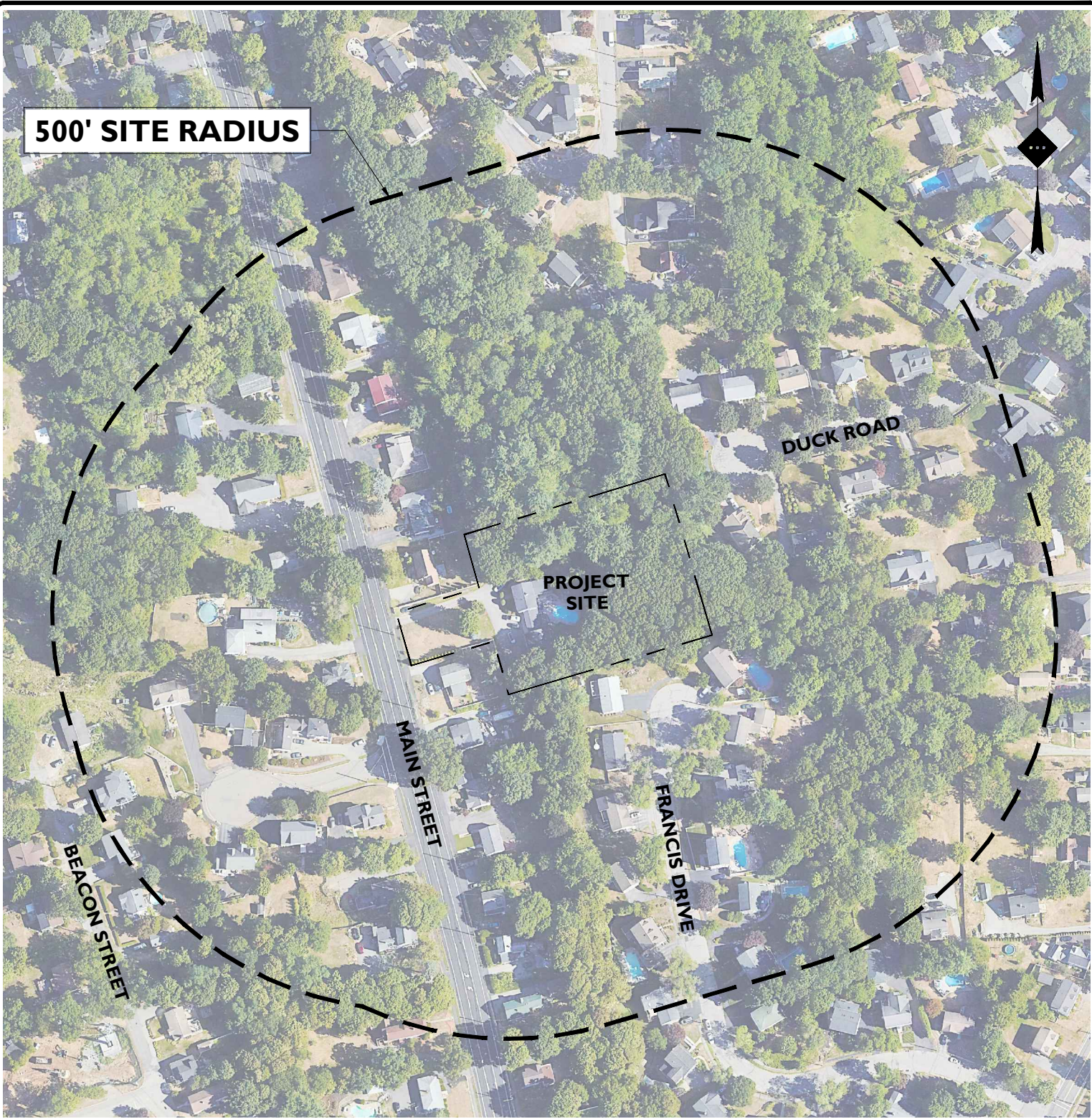
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500' SITE RADIUS



PROJECT SITE

DUCK ROAD

MAIN STREET

FRANCIS DRIVE

BEACON STREET



GRAPHIC SCALE IN FEET

1" = 200'

AERIAL MAP

SOURCE: GOOGLE EARTH IMAGE, DATED 06/13/2024

PRIMROSE SCHOOLS FRANCHISING COMPANY PROPOSED CHILD DAY CARE FACILITY

PARCEL ID: 28-113
885 MAIN STREET, TOWN OF READING
MIDDLESEX COUNTY, MASSACHUSETTS



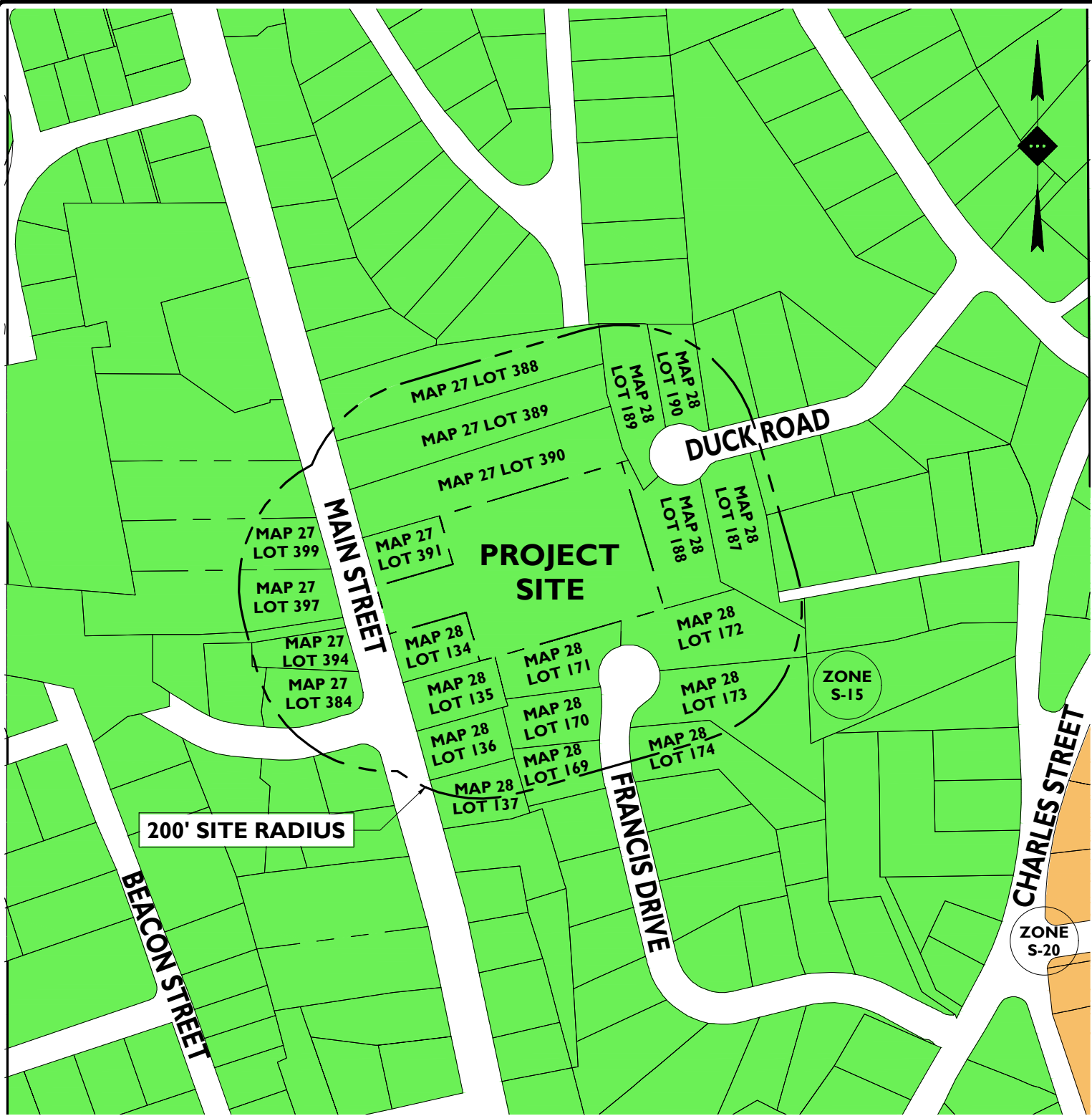
DRAWN BY:	SCL
CHECKED BY:	JHK
DATE:	02/27/2025
SCALE:	1" = 200'
PROJECT ID:	BOS-240115



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TAX & ZONING MAP



GRAPHIC SCALE IN FEET
1" = 200'

SOURCE: TOWN OF READING ZONING MAP DATED 04/27/2025 & TOWN OF READING MAPGEO

PRIMROSE SCHOOLS FRANCHISING COMPANY PROPOSED CHILD DAY CARE FACILITY

PARCEL ID: 28-113
885 MAIN STREET, TOWN OF READING
MIDDLESEX COUNTY, MASSACHUSETTS



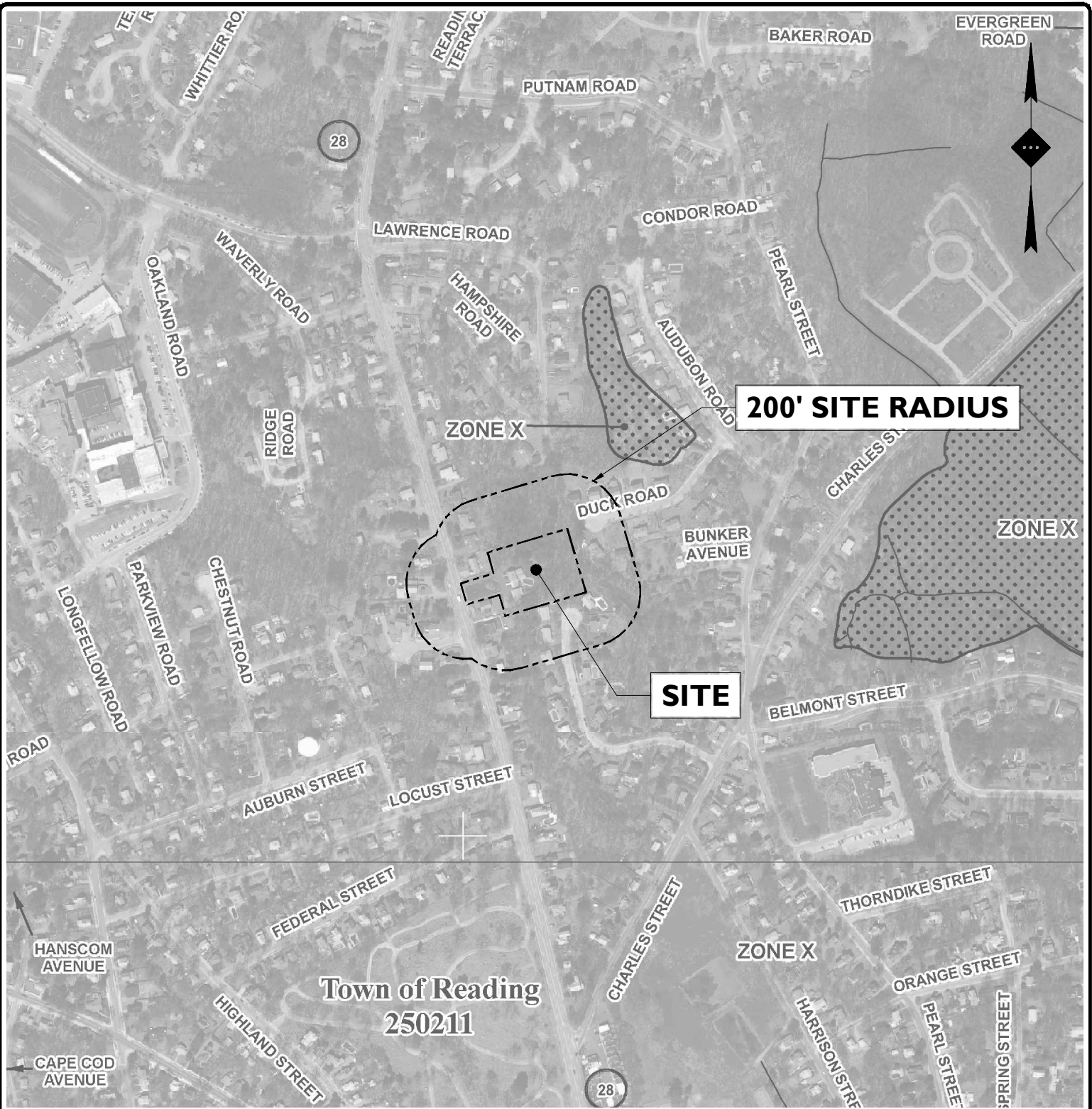
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CHECKED BY:	JHK
DATE:	02/27/2025
SCALE:	1" = 200'
PROJECT ID:	BOS-240115



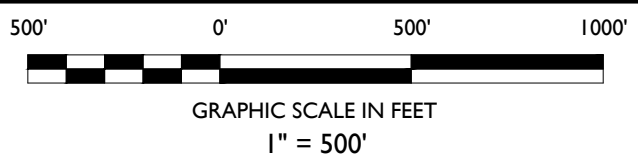
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FEMA FLOOD MAP



SOURCE: FEMA FLOOD MAP NUMBER 25017C0311E & 25017C0313E

PRIMROSE SCHOOLS FRANCHISING COMPANY
PROPOSED CHILD DAY CARE FACILITY



PARCEL ID: 28-113
 885 MAIN STREET, TOWN OF READING
 MIDDLESEX COUNTY, MASSACHUSETTS

DRAWN BY:	SCL
CHECKED BY:	JHK
DATE:	02/27/2025
SCALE:	1" = 500'
PROJECT ID:	BOS-240115

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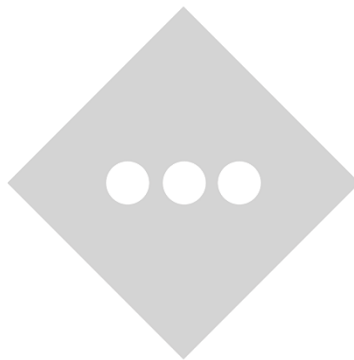
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APPENDIX B PROJECT SOILS

INVENTORY

B-1: NRCS SOILS REPORT





United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

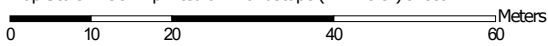
Custom Soil Resource Report for Middlesex County, Massachusetts



Custom Soil Resource Report Soil Map



Map Scale: 1:934 if printed on A landscape (11" x 8.5") sheet.




Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)


Soils


 Soil Map Unit Polygons


 Soil Map Unit Lines


 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit


 Clay Spot


 Closed Depression

 Gravel Pit


 Gravelly Spot


 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water


 Perennial Water

 Rock Outcrop


 Saline Spot

 Sandy Spot

 Severely Eroded Spot


 Sinkhole

 Slide or Slip


 Sodic Spot


 Spoil Area

 Stony Spot


 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals


Transportation

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Middlesex County, Massachusetts
 Survey Area Data: Version 24, Aug 27, 2024

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Mar 1, 2023—Sep 1, 2023

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
73B	Whitman fine sandy loam, 0 to 3 percent slopes, extremely stony	1.3	53.0%
305C	Paxton fine sandy loam, 8 to 15 percent slopes	0.1	3.8%
629C	Canton-Charlton-Urban land complex, 3 to 15 percent slopes	0.0	0.5%
631C	Charlton-Urban land-Hollis complex, 3 to 15 percent slopes, rocky	0.9	37.5%
655	Udorthents, wet substratum	0.1	5.2%
Totals for Area of Interest		2.4	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it

Custom Soil Resource Report

was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Middlesex County, Massachusetts

73B—Whitman fine sandy loam, 0 to 3 percent slopes, extremely stony

Map Unit Setting

National map unit symbol: 2w695
Elevation: 0 to 1,580 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Whitman, extremely stony, and similar soils: 81 percent
Minor components: 19 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Whitman, Extremely Stony

Setting

Landform: Drumlins, ground moraines, hills, drainageways, depressions
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Base slope
Down-slope shape: Concave
Across-slope shape: Concave
Parent material: Coarse-loamy lodgment till derived from gneiss, granite, and/or schist

Typical profile

O_i - 0 to 1 inches: peat
A - 1 to 10 inches: fine sandy loam
B_g - 10 to 17 inches: gravelly fine sandy loam
C_{dg} - 17 to 61 inches: fine sandy loam

Properties and qualities

Slope: 0 to 3 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: 7 to 38 inches to densic material
Drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (K_{sat}): Very low to moderately low (0.00 to 0.14 in/hr)
Depth to water table: About 0 to 6 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7s
Hydrologic Soil Group: D
Ecological site: F144AY041MA - Very Wet Till Depressions
Hydric soil rating: Yes

Minor Components

Ridgebury, extremely stony

Percent of map unit: 10 percent
Landform: Drumlins, depressions, ground moraines, hills, drainageways
Landform position (two-dimensional): Footslope, toeslope
Landform position (three-dimensional): Head slope, base slope
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Scarboro

Percent of map unit: 5 percent
Landform: Drainageways, depressions, outwash terraces, outwash deltas
Landform position (three-dimensional): Tread
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Swansea

Percent of map unit: 3 percent
Landform: Marshes, bogs, swamps
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Woodbridge, extremely stony

Percent of map unit: 1 percent
Landform: Ground moraines, hills, drumlins
Landform position (two-dimensional): Summit, backslope, footslope
Landform position (three-dimensional): Side slope, crest
Down-slope shape: Concave
Across-slope shape: Linear
Hydric soil rating: No

305C—Paxton fine sandy loam, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 2w66y
Elevation: 0 to 1,320 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Farmland of statewide importance

Map Unit Composition

Paxton and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Custom Soil Resource Report

Description of Paxton

Setting

Landform: Ground moraines, hills, drumlins
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Convex, linear
Across-slope shape: Convex
Parent material: Coarse-loamy lodgment till derived from gneiss, granite, and/or schist

Typical profile

Ap - 0 to 8 inches: fine sandy loam
Bw1 - 8 to 15 inches: fine sandy loam
Bw2 - 15 to 26 inches: fine sandy loam
Cd - 26 to 65 inches: gravelly fine sandy loam

Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: 20 to 39 inches to densic material
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.14 in/hr)
Depth to water table: About 18 to 37 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 4.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: C
Ecological site: F144AY007CT - Well Drained Dense Till Uplands
Hydric soil rating: No

Minor Components

Charlton

Percent of map unit: 7 percent
Landform: Hills
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Convex
Across-slope shape: Convex
Hydric soil rating: No

Woodbridge

Percent of map unit: 6 percent
Landform: Hills, drumlins, ground moraines
Landform position (two-dimensional): Summit, backslope, footslope
Landform position (three-dimensional): Side slope
Down-slope shape: Concave
Across-slope shape: Linear
Hydric soil rating: No

Ridgebury

Percent of map unit: 2 percent
Landform: Drumlins, drainageways, depressions, ground moraines, hills
Landform position (two-dimensional): Footslope, toeslope
Landform position (three-dimensional): Head slope, base slope
Down-slope shape: Concave, linear
Across-slope shape: Concave, linear
Hydric soil rating: Yes

629C—Canton-Charlton-Urban land complex, 3 to 15 percent slopes

Map Unit Setting

National map unit symbol: 9959
Elevation: 0 to 1,000 feet
Mean annual precipitation: 32 to 54 inches
Mean annual air temperature: 43 to 54 degrees F
Frost-free period: 110 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Canton and similar soils: 40 percent
Charlton and similar soils: 30 percent
Urban land: 25 percent
Minor components: 5 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Canton

Setting

Landform: Hills
Landform position (two-dimensional): Backslope, footslope
Landform position (three-dimensional): Side slope, base slope
Down-slope shape: Linear
Across-slope shape: Convex
Parent material: Friable loamy eolian deposits over friable sandy basal till derived from granite and gneiss

Typical profile

H1 - 0 to 8 inches: fine sandy loam
H2 - 8 to 21 inches: fine sandy loam
H3 - 21 to 65 inches: gravelly loamy sand

Properties and qualities

Slope: 3 to 15 percent
Depth to restrictive feature: 18 to 30 inches to strongly contrasting textural stratification
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)
Depth to water table: More than 80 inches

Custom Soil Resource Report

Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Very low (about 2.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: A
Ecological site: F144AY034CT - Well Drained Till Uplands
Hydric soil rating: No

Description of Charlton

Setting

Landform: Ground moraines, drumlins
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Convex
Parent material: Friable loamy eolian deposits over friable loamy basal till derived from granite and gneiss

Typical profile

H1 - 0 to 5 inches: fine sandy loam
H2 - 5 to 22 inches: sandy loam
H3 - 22 to 65 inches: gravelly sandy loam

Properties and qualities

Slope: 3 to 15 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Moderate (about 7.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: A
Ecological site: F144AY034CT - Well Drained Till Uplands
Hydric soil rating: No

Description of Urban Land

Setting

Landform position (two-dimensional): Footslope
Landform position (three-dimensional): Base slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Excavated and filled land

Minor Components

Scituate

Percent of map unit: 2 percent

Custom Soil Resource Report

Landform: Hillslopes, depressions
Landform position (two-dimensional): Summit, toeslope
Landform position (three-dimensional): Head slope, base slope
Down-slope shape: Linear
Across-slope shape: Concave
Hydric soil rating: No

Montauk

Percent of map unit: 2 percent
Landform: Hillslopes
Landform position (two-dimensional): Summit, shoulder
Landform position (three-dimensional): Head slope, nose slope
Down-slope shape: Convex
Across-slope shape: Convex
Hydric soil rating: No

Udorthents, loamy

Percent of map unit: 1 percent
Hydric soil rating: No

631C—Charlton-Urban land-Hollis complex, 3 to 15 percent slopes, rocky

Map Unit Setting

National map unit symbol: vr1g
Elevation: 0 to 1,000 feet
Mean annual precipitation: 32 to 54 inches
Mean annual air temperature: 43 to 54 degrees F
Frost-free period: 110 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Charlton and similar soils: 45 percent
Urban land: 35 percent
Hollis and similar soils: 10 percent
Minor components: 10 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Charlton

Setting

Landform: Ground moraines, drumlins
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Convex
Parent material: Friable loamy eolian deposits over friable loamy basal till derived from granite and gneiss

Custom Soil Resource Report

Typical profile

H1 - 0 to 5 inches: fine sandy loam

H2 - 5 to 22 inches: sandy loam

H3 - 22 to 65 inches: gravelly sandy loam

Properties and qualities

Slope: 3 to 15 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high
(0.60 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 7.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: A

Ecological site: F144AY034CT - Well Drained Till Uplands

Hydric soil rating: No

Description of Urban Land

Setting

Landform position (two-dimensional): Footslope

Landform position (three-dimensional): Base slope

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Excavated and filled land

Description of Hollis

Setting

Landform: Hillslopes, ridges

Landform position (two-dimensional): Backslope

Landform position (three-dimensional): Side slope

Down-slope shape: Linear

Across-slope shape: Convex

Parent material: Friable, shallow loamy basal till over granite and gneiss

Typical profile

H1 - 0 to 2 inches: fine sandy loam

H2 - 2 to 14 inches: fine sandy loam

H3 - 14 to 18 inches: unweathered bedrock

Properties and qualities

Slope: 3 to 15 percent

Surface area covered with cobbles, stones or boulders: 9.0 percent

Depth to restrictive feature: 8 to 20 inches to lithic bedrock

Drainage class: Well drained

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately
low (0.00 to 0.14 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Custom Soil Resource Report

Available water supply, 0 to 60 inches: Very low (about 2.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6s

Hydrologic Soil Group: D

Ecological site: F144AY033MA - Shallow Dry Till Uplands

Hydric soil rating: No

Minor Components

Canton

Percent of map unit: 4 percent

Landform: Hills

Landform position (two-dimensional): Backslope, toeslope

Landform position (three-dimensional): Side slope, base slope

Down-slope shape: Linear

Across-slope shape: Convex

Hydric soil rating: No

Udorthents, loamy

Percent of map unit: 2 percent

Hydric soil rating: No

Rock outcrop

Percent of map unit: 2 percent

Landform: Ledges

Landform position (two-dimensional): Summit

Landform position (three-dimensional): Head slope

Down-slope shape: Concave

Across-slope shape: Concave

Scituate

Percent of map unit: 1 percent

Landform: Hillslopes, depressions

Landform position (two-dimensional): Summit, toeslope

Landform position (three-dimensional): Head slope, base slope

Down-slope shape: Linear

Across-slope shape: Concave

Hydric soil rating: No

Montauk

Percent of map unit: 1 percent

Landform: Hillslopes

Landform position (two-dimensional): Summit, shoulder

Landform position (three-dimensional): Head slope, nose slope

Down-slope shape: Convex

Across-slope shape: Convex

Hydric soil rating: No

655—Udorthents, wet substratum

Map Unit Setting

National map unit symbol: vr1n
Elevation: 0 to 3,000 feet
Mean annual precipitation: 32 to 54 inches
Mean annual air temperature: 43 to 54 degrees F
Frost-free period: 110 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Udorthents, wet substratum, and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Udorthents, Wet Substratum

Setting

Parent material: Loamy alluvium and/or sandy glaciofluvial deposits and/or loamy glaciolacustrine deposits and/or loamy marine deposits and/or loamy basal till and/or loamy lodgment till

Properties and qualities

Slope: 0 to 8 percent
Depth to restrictive feature: More than 80 inches
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None

Minor Components

Urban land

Percent of map unit: 8 percent
Landform position (two-dimensional): Footslope
Landform position (three-dimensional): Base slope
Down-slope shape: Linear
Across-slope shape: Linear

Freetown

Percent of map unit: 4 percent
Landform: Depressions, bogs
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Custom Soil Resource Report

Swansea

Percent of map unit: 3 percent

Landform: Depressions, bogs

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Dip

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: Yes

References

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Custom Soil Resource Report

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf

APPENDIX C

HYDROLOGIC & HYDRAULIC CALCULATIONS

INVENTORY

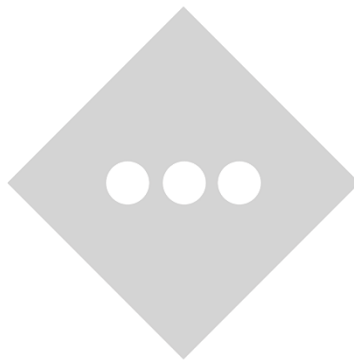
C-1: TSS REMOVAL CALCULATIONS

C-2: HYDROCAD NODE SCHEMATIC DIAGRAM

C-3: HYDROCAD HYDROLOGIC CALCULATIONS

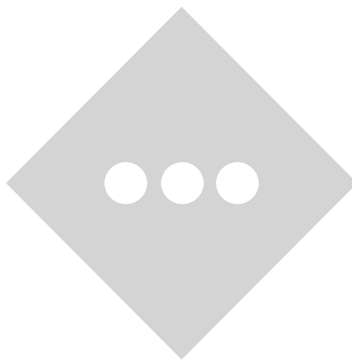
C-4: INFILTRATION BASIN STAGE-STORAGE TABLES

C-5: INFILTRATION BASIN STAGE-DISCHARGE TABLES



APPENDIX C-I

TSS REMOVAL CALCULATIONS



INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location:

TSS Removal Calculation Worksheet

	B BMP ¹	C TSS Removal Rate ¹	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
	Street Sweeping - 9%	0.09	1.00	0.09	0.91
	Deep Sump and Hooded Catch Basin	0.25	0.91	0.23	0.68
	Isolator Row	0.25	0.68	0.17	0.51
	Infiltration Basin	0.80	0.51	0.41	0.10
		0.00	0.10	0.00	0.10

Total TSS Removal =

Separate Form Needs to be Completed for Each Outlet or BMP Train

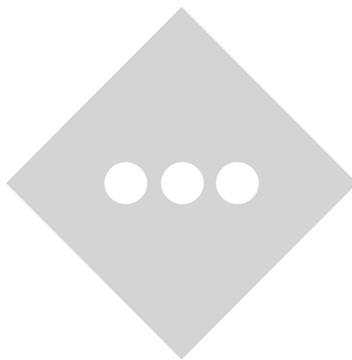
Project:
 Prepared By:
 Date:

*Equals remaining load from previous BMP (E) which enters the BMP

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed
 1. From MassDEP Stormwater Handbook Vol. 1

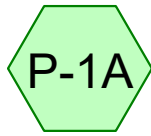
APPENDIX C-2

HYDROCAD NODE SCHEMATIC DIAGRAM

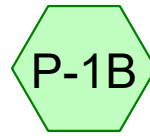




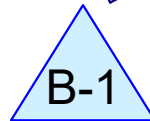
Runoff to Wetlands



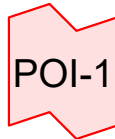
Direct to Wetlands



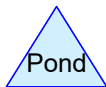
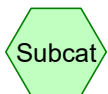
Parking Lot



StormTech SC-800
Subsurface Infiltration
System



Wetlands

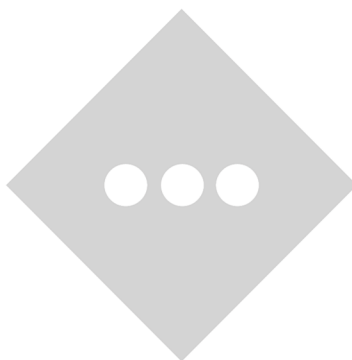


Routing Diagram for 2025-2-28 HydroCAD

Prepared by Stonefield Engineering & Design, Printed 3/6/2025
HydroCAD® 10.20-6a s/n 10626 © 2024 HydroCAD Software Solutions LLC

APPENDIX C-3

HYDROCAD HYDROLOGIC CALCULATIONS



Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentEX-1: Runoff to Wetlands Runoff Area=84,280 sf 13.70% Impervious Runoff Depth=0.97"
Flow Length=204' Tc=11.2 min CN=68/98 Runoff=1.49 cfs 6,833 cf

SubcatchmentP-1A: Direct to Wetlands Runoff Area=54,756 sf 3.83% Impervious Runoff Depth=0.94"
Flow Length=238' Tc=6.6 min CN=72/98 Runoff=1.21 cfs 4,282 cf

SubcatchmentP-1B: Parking Lot Runoff Area=29,525 sf 97.99% Impervious Runoff Depth=2.81"
Tc=6.0 min CN=66/98 Runoff=1.85 cfs 6,920 cf

Pond B-1: StormTech SC-800 Subsurface Peak Elev=106.21' Storage=1,844 cf Inflow=1.85 cfs 6,920 cf
Discarded=0.22 cfs 6,920 cf Primary=0.00 cfs 0 cf Outflow=0.22 cfs 6,920 cf

Link POI-1: Wetlands Inflow=1.21 cfs 4,282 cf
Primary=1.21 cfs 4,282 cf

Total Runoff Area = 168,561 sf Runoff Volume = 18,035 cf Average Runoff Depth = 1.28"
74.74% Pervious = 125,986 sf 25.26% Impervious = 42,575 sf

Summary for Subcatchment EX-1: Runoff to Wetlands

Runoff = 1.49 cfs @ 12.19 hrs, Volume= 6,833 cf, Depth= 0.97"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
NRCC 24-hr D 2-Year Rainfall=3.09"

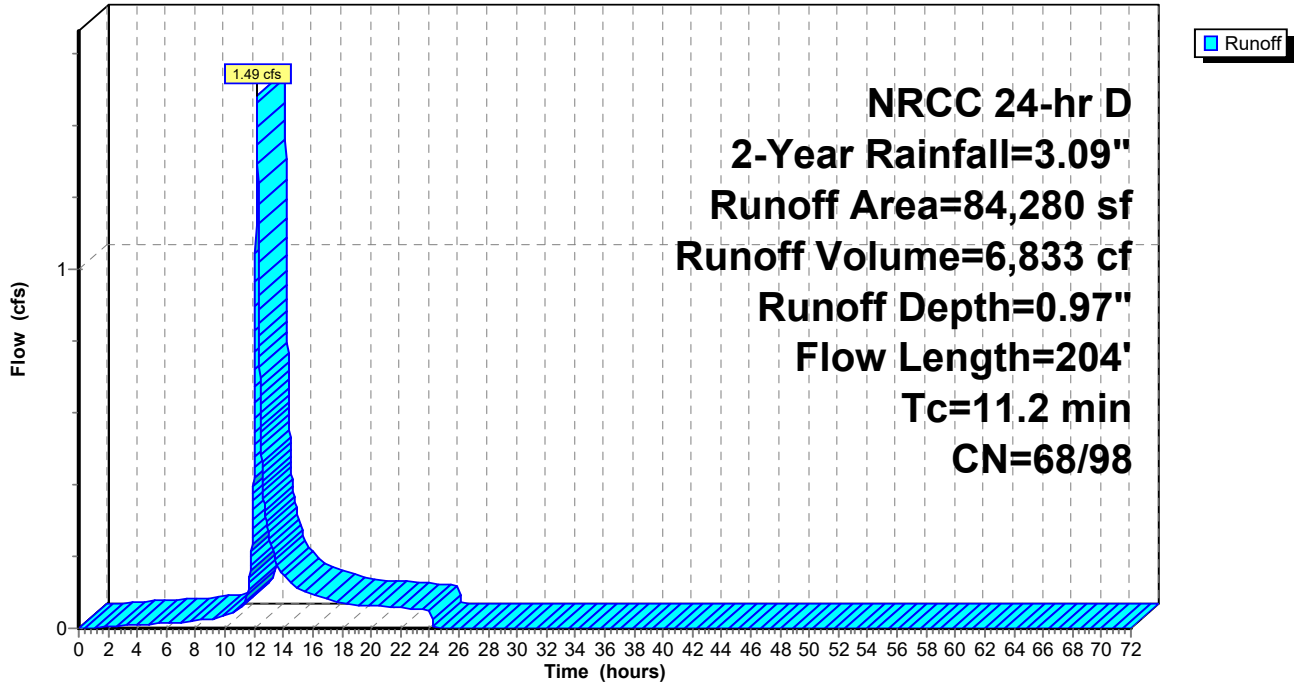
Area (sf)	CN	Description
11,547	98	Unconnected pavement, HSG D
7,989	30	Woods, Good, HSG A
44,941	77	Woods, Good, HSG D
5,755	80	>75% Grass cover, Good, HSG D
7,174	74	>75% Grass cover, Good, HSG C
6,874	39	>75% Grass cover, Good, HSG A

84,280	72	Weighted Average
72,733	68	86.30% Pervious Area
11,547	98	13.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.3	96	0.1200	0.15		Sheet Flow, 1A-1B Woods: Light underbrush n= 0.400 P2= 3.09"
0.6	63	0.0670	1.81		Shallow Concentrated Flow, 1B-1C Short Grass Pasture Kv= 7.0 fps
0.3	45	0.2000	2.24		Shallow Concentrated Flow, 1C-1D Woodland Kv= 5.0 fps
11.2	204	Total			

Subcatchment EX-1: Runoff to Wetlands

Hydrograph



Summary for Subcatchment P-1A: Direct to Wetlands

Runoff = 1.21 cfs @ 12.14 hrs, Volume= 4,282 cf, Depth= 0.94"
 Routed to Link POI-1 : Wetlands

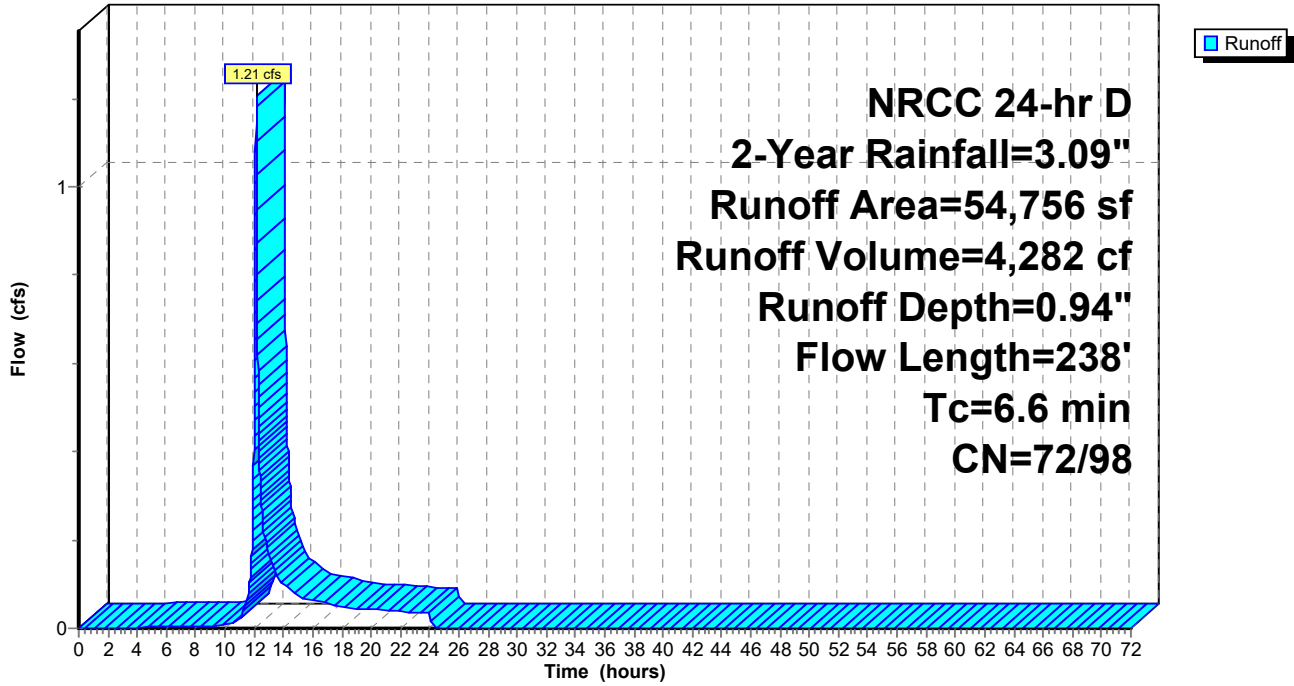
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 NRCC 24-hr D 2-Year Rainfall=3.09"

Area (sf)	CN	Description
* 2,095	98	Impervious
* 582	39	Turf Area, HSG A
* 8,718	80	Turf Area, HSG D
1,088	30	Woods, Good, HSG A
21,265	77	Woods, Good, HSG D
6,416	39	>75% Grass cover, Good, HSG A
674	74	>75% Grass cover, Good, HSG C
13,918	80	>75% Grass cover, Good, HSG D
54,756	73	Weighted Average
52,661	72	96.17% Pervious Area
2,095	98	3.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	100	0.0850	0.30		Sheet Flow, 1A-1B Grass: Short n= 0.150 P2= 3.09"
0.9	105	0.0140	1.90		Shallow Concentrated Flow, 1B-1C Unpaved Kv= 16.1 fps
0.1	33	0.0600	3.94		Shallow Concentrated Flow, 1C-1D Unpaved Kv= 16.1 fps
6.6	238	Total			

Subcatchment P-1A: Direct to Wetlands

Hydrograph



Summary for Subcatchment P-1B: Parking Lot

Runoff = 1.85 cfs @ 12.13 hrs, Volume= 6,920 cf, Depth= 2.81"

Routed to Pond B-1 : StormTech SC-800 Subsurface Infiltration System

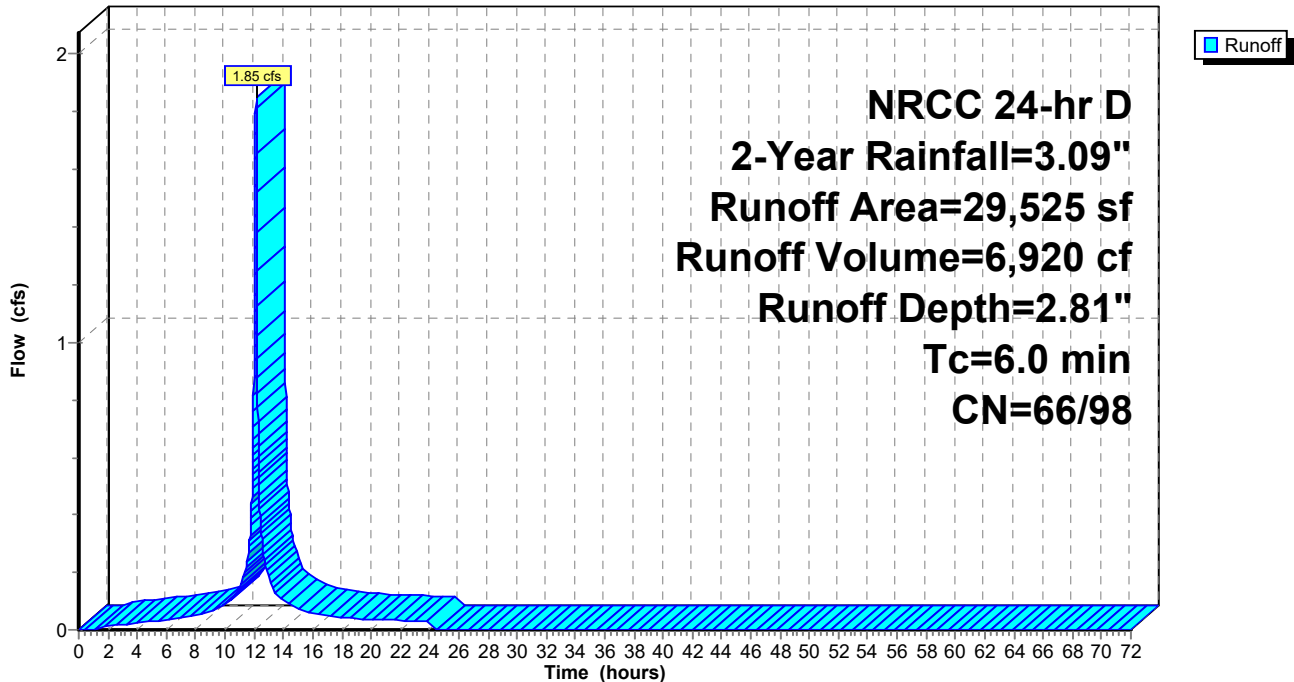
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
NRCC 24-hr D 2-Year Rainfall=3.09"

	Area (sf)	CN	Description
*	28,933	98	Impervious
	238	80	>75% Grass cover, Good, HSG D
	181	74	>75% Grass cover, Good, HSG C
	173	39	>75% Grass cover, Good, HSG A
	29,525	97	Weighted Average
	592	66	2.01% Pervious Area
	28,933	98	97.99% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min

Subcatchment P-1B: Parking Lot

Hydrograph



Summary for Pond B-1: StormTech SC-800 Subsurface Infiltration System

Inflow Area = 29,525 sf, 97.99% Impervious, Inflow Depth = 2.81" for 2-Year event
 Inflow = 1.85 cfs @ 12.13 hrs, Volume= 6,920 cf
 Outflow = 0.22 cfs @ 12.76 hrs, Volume= 6,920 cf, Atten= 88%, Lag= 38.2 min
 Discarded = 0.22 cfs @ 12.76 hrs, Volume= 6,920 cf
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
 Routed to Link POI-1 : Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 Peak Elev= 106.21' @ 12.76 hrs Surf.Area= 2,357 sf Storage= 1,844 cf

Plug-Flow detention time= 54.7 min calculated for 6,919 cf (100% of inflow)
 Center-of-Mass det. time= 54.7 min (816.7 - 762.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	105.00'	2,241 cf	34.75"W x 67.82"L x 3.75"H Field A 8,837 cf Overall - 3,235 cf Embedded = 5,602 cf x 40.0% Voids
#2A	105.50'	3,235 cf	ADS_StormTech SC-800 +Cap x 63 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 63 Chambers in 7 Rows Cap Storage= 3.4 cf x 2 x 7 rows = 47.9 cf
		5,476 cf	Total Available Storage

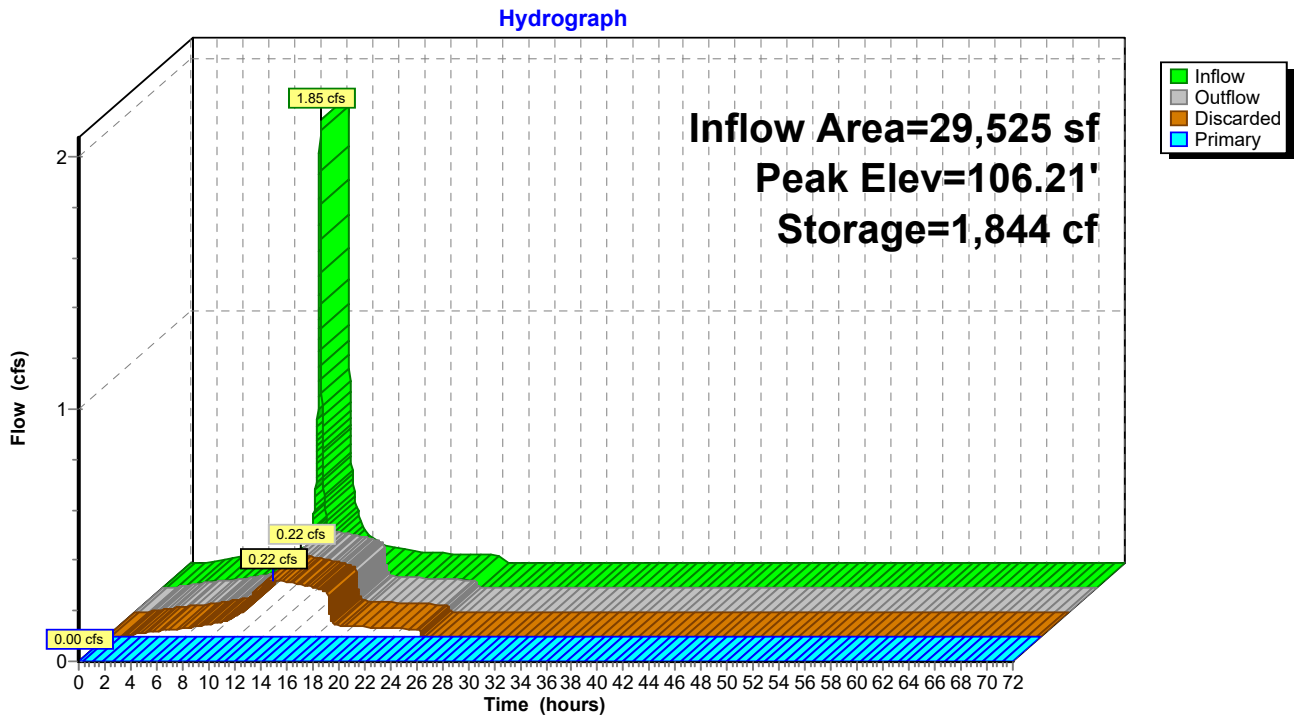
Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	104.00'	12.0" Round Culvert L= 45.0' Ke= 0.500 Inlet / Outlet Invert= 104.00' / 103.50' S= 0.0111 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	107.10'	6.0" W x 4.0" H Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	108.20'	6.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#4	Discarded	105.00'	3.150 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 101.00' Phase-In= 0.10'

Discarded OutFlow Max=0.22 cfs @ 12.76 hrs HW=106.21' (Free Discharge)
 ↳4=Exfiltration (Controls 0.22 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=105.00' TW=0.00' (Dynamic Tailwater)
 ↳1=Culvert (Passes 0.00 cfs of 2.67 cfs potential flow)
 ↳2=Orifice/Grate (Controls 0.00 cfs)
 ↳3=Broad-Crested Rectangular Weir(Controls 0.00 cfs)

Pond B-1: StormTech SC-800 Subsurface Infiltration System



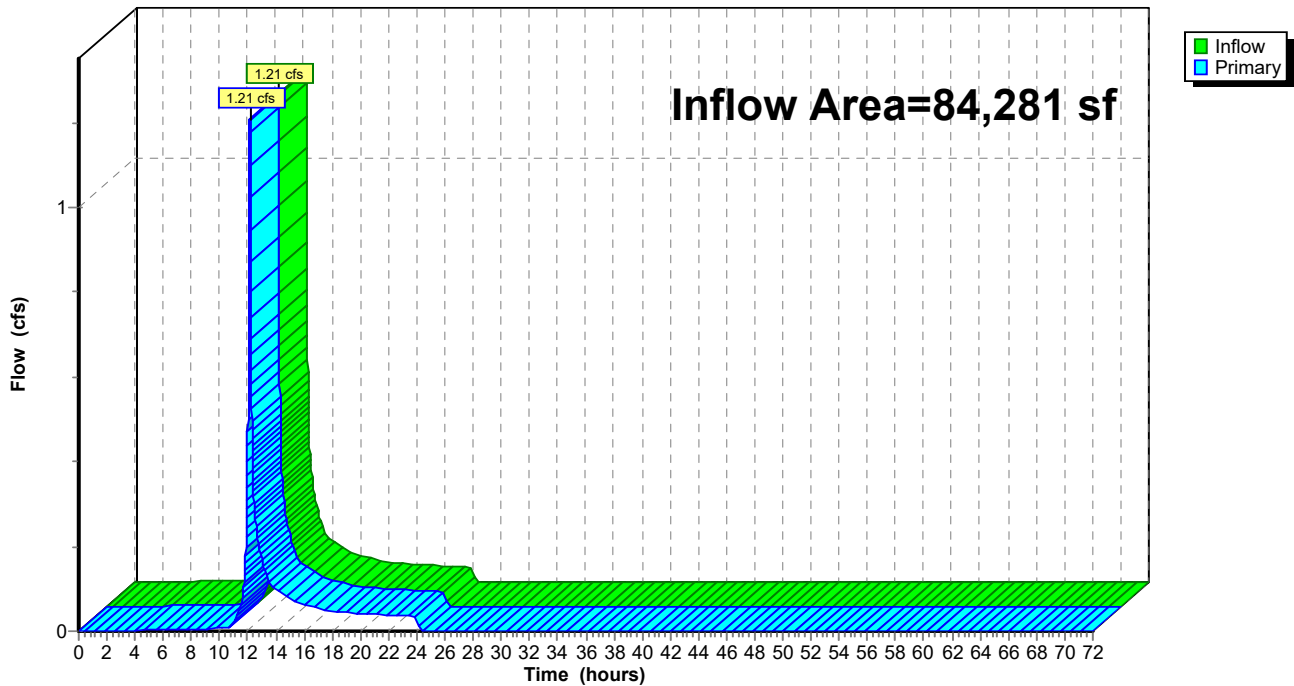
Summary for Link POI-1: Wetlands

Inflow Area = 84,281 sf, 36.81% Impervious, Inflow Depth = 0.61" for 2-Year event
Inflow = 1.21 cfs @ 12.14 hrs, Volume= 4,282 cf
Primary = 1.21 cfs @ 12.14 hrs, Volume= 4,282 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link POI-1: Wetlands

Hydrograph



Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentEX-1: Runoff to Wetlands Runoff Area=84,280 sf 13.70% Impervious Runoff Depth=2.02"
Flow Length=204' Tc=11.2 min CN=68/98 Runoff=3.34 cfs 14,155 cf

SubcatchmentP-1A: Direct to Wetlands Runoff Area=54,756 sf 3.83% Impervious Runoff Depth=2.03"
Flow Length=238' Tc=6.6 min CN=72/98 Runoff=2.74 cfs 9,249 cf

SubcatchmentP-1B: Parking Lot Runoff Area=29,525 sf 97.99% Impervious Runoff Depth=4.36"
Tc=6.0 min CN=66/98 Runoff=2.82 cfs 10,716 cf

Pond B-1: StormTech SC-800 Subsurface Peak Elev=107.07' Storage=3,379 cf Inflow=2.82 cfs 10,716 cf
Discarded=0.26 cfs 10,716 cf Primary=0.00 cfs 0 cf Outflow=0.26 cfs 10,716 cf

Link POI-1: Wetlands Inflow=2.74 cfs 9,249 cf
Primary=2.74 cfs 9,249 cf

Total Runoff Area = 168,561 sf Runoff Volume = 34,119 cf Average Runoff Depth = 2.43"
74.74% Pervious = 125,986 sf 25.26% Impervious = 42,575 sf

Summary for Subcatchment EX-1: Runoff to Wetlands

Runoff = 3.34 cfs @ 12.19 hrs, Volume= 14,155 cf, Depth= 2.02"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
NRCC 24-hr D 10-Year Rainfall=4.65"

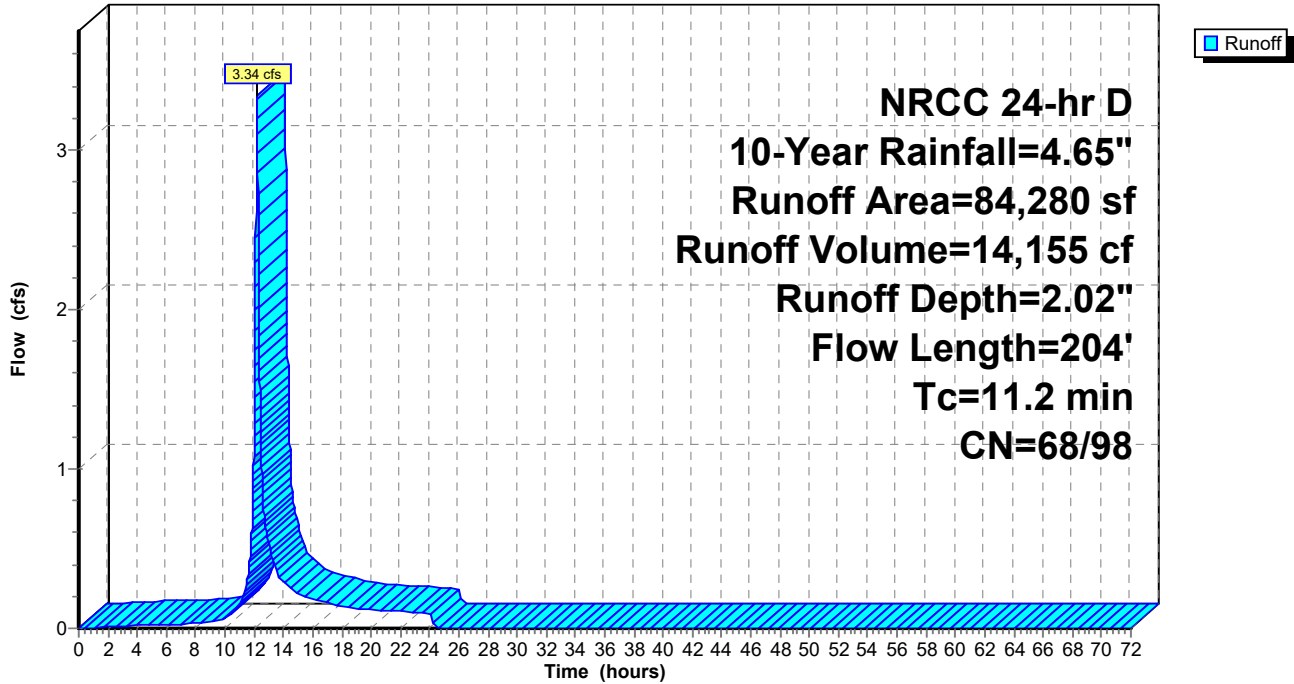
Area (sf)	CN	Description
11,547	98	Unconnected pavement, HSG D
7,989	30	Woods, Good, HSG A
44,941	77	Woods, Good, HSG D
5,755	80	>75% Grass cover, Good, HSG D
7,174	74	>75% Grass cover, Good, HSG C
6,874	39	>75% Grass cover, Good, HSG A

84,280	72	Weighted Average
72,733	68	86.30% Pervious Area
11,547	98	13.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.3	96	0.1200	0.15		Sheet Flow, 1A-1B Woods: Light underbrush n= 0.400 P2= 3.09"
0.6	63	0.0670	1.81		Shallow Concentrated Flow, 1B-1C Short Grass Pasture Kv= 7.0 fps
0.3	45	0.2000	2.24		Shallow Concentrated Flow, 1C-1D Woodland Kv= 5.0 fps
11.2	204	Total			

Subcatchment EX-1: Runoff to Wetlands

Hydrograph



Summary for Subcatchment P-1A: Direct to Wetlands

Runoff = 2.74 cfs @ 12.14 hrs, Volume= 9,249 cf, Depth= 2.03"
 Routed to Link POI-1 : Wetlands

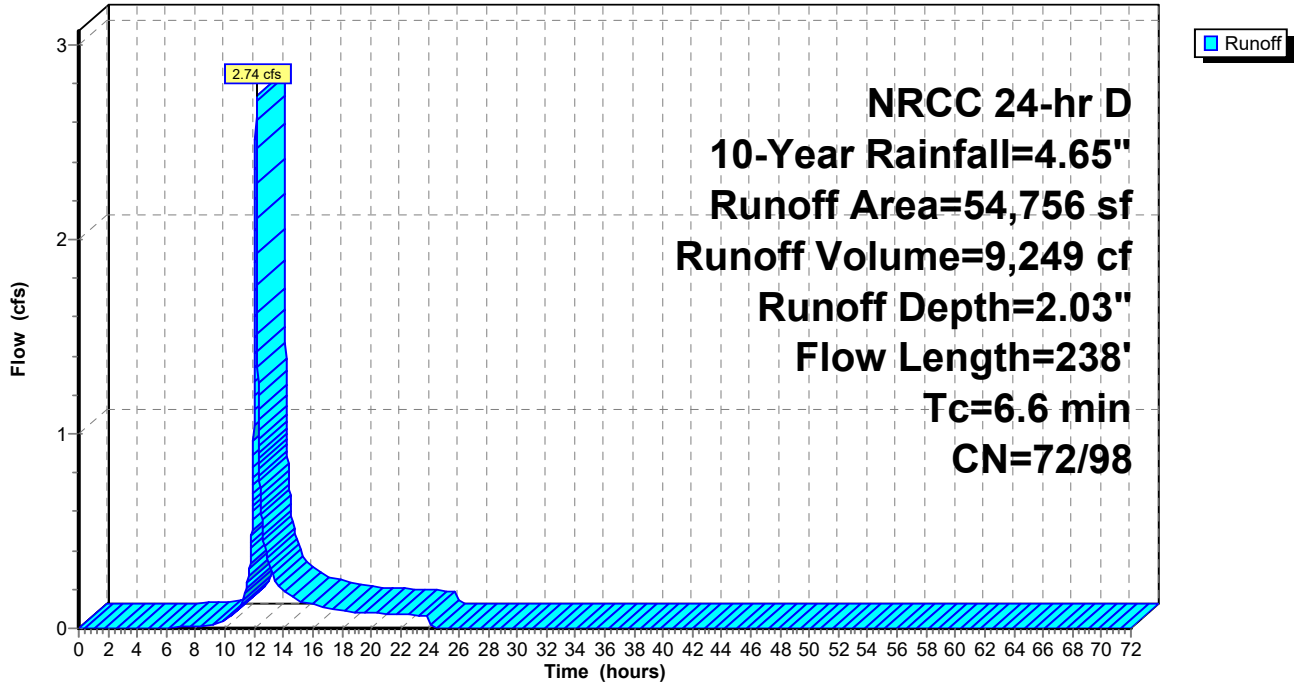
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 NRCC 24-hr D 10-Year Rainfall=4.65"

Area (sf)	CN	Description
* 2,095	98	Impervious
* 582	39	Turf Area, HSG A
* 8,718	80	Turf Area, HSG D
1,088	30	Woods, Good, HSG A
21,265	77	Woods, Good, HSG D
6,416	39	>75% Grass cover, Good, HSG A
674	74	>75% Grass cover, Good, HSG C
13,918	80	>75% Grass cover, Good, HSG D
54,756	73	Weighted Average
52,661	72	96.17% Pervious Area
2,095	98	3.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	100	0.0850	0.30		Sheet Flow, 1A-1B Grass: Short n= 0.150 P2= 3.09"
0.9	105	0.0140	1.90		Shallow Concentrated Flow, 1B-1C Unpaved Kv= 16.1 fps
0.1	33	0.0600	3.94		Shallow Concentrated Flow, 1C-1D Unpaved Kv= 16.1 fps
6.6	238	Total			

Subcatchment P-1A: Direct to Wetlands

Hydrograph



Summary for Subcatchment P-1B: Parking Lot

Runoff = 2.82 cfs @ 12.13 hrs, Volume= 10,716 cf, Depth= 4.36"

Routed to Pond B-1 : StormTech SC-800 Subsurface Infiltration System

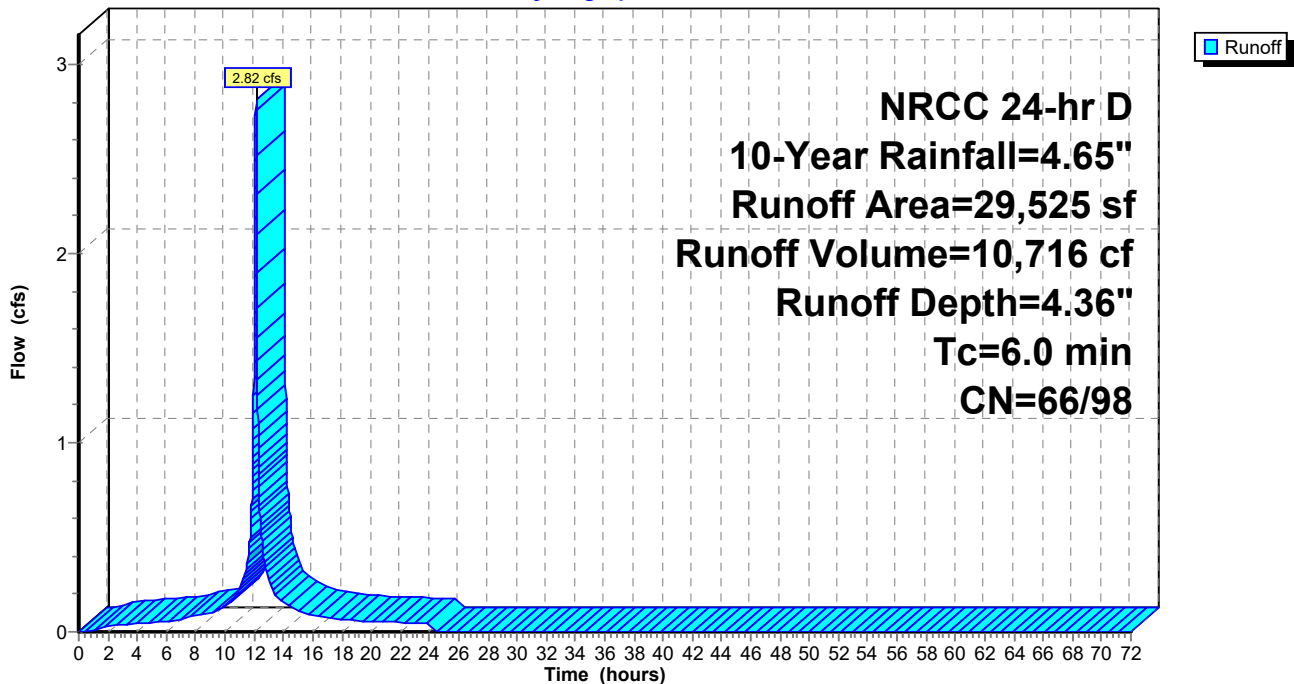
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
NRCC 24-hr D 10-Year Rainfall=4.65"

	Area (sf)	CN	Description
*	28,933	98	Impervious
	238	80	>75% Grass cover, Good, HSG D
	181	74	>75% Grass cover, Good, HSG C
	173	39	>75% Grass cover, Good, HSG A
	29,525	97	Weighted Average
	592	66	2.01% Pervious Area
	28,933	98	97.99% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min

Subcatchment P-1B: Parking Lot

Hydrograph



Summary for Pond B-1: StormTech SC-800 Subsurface Infiltration System

Inflow Area = 29,525 sf, 97.99% Impervious, Inflow Depth = 4.36" for 10-Year event
 Inflow = 2.82 cfs @ 12.13 hrs, Volume= 10,716 cf
 Outflow = 0.26 cfs @ 13.08 hrs, Volume= 10,716 cf, Atten= 91%, Lag= 57.0 min
 Discarded = 0.26 cfs @ 13.08 hrs, Volume= 10,716 cf
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
 Routed to Link POI-1 : Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 Peak Elev= 107.07' @ 13.08 hrs Surf.Area= 2,357 sf Storage= 3,379 cf

Plug-Flow detention time= 97.8 min calculated for 10,714 cf (100% of inflow)
 Center-of-Mass det. time= 97.8 min (850.7 - 752.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	105.00'	2,241 cf	34.75"W x 67.82'L x 3.75'H Field A 8,837 cf Overall - 3,235 cf Embedded = 5,602 cf x 40.0% Voids
#2A	105.50'	3,235 cf	ADS_StormTech SC-800 +Cap x 63 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 63 Chambers in 7 Rows Cap Storage= 3.4 cf x 2 x 7 rows = 47.9 cf
		5,476 cf	Total Available Storage

Storage Group A created with Chamber Wizard

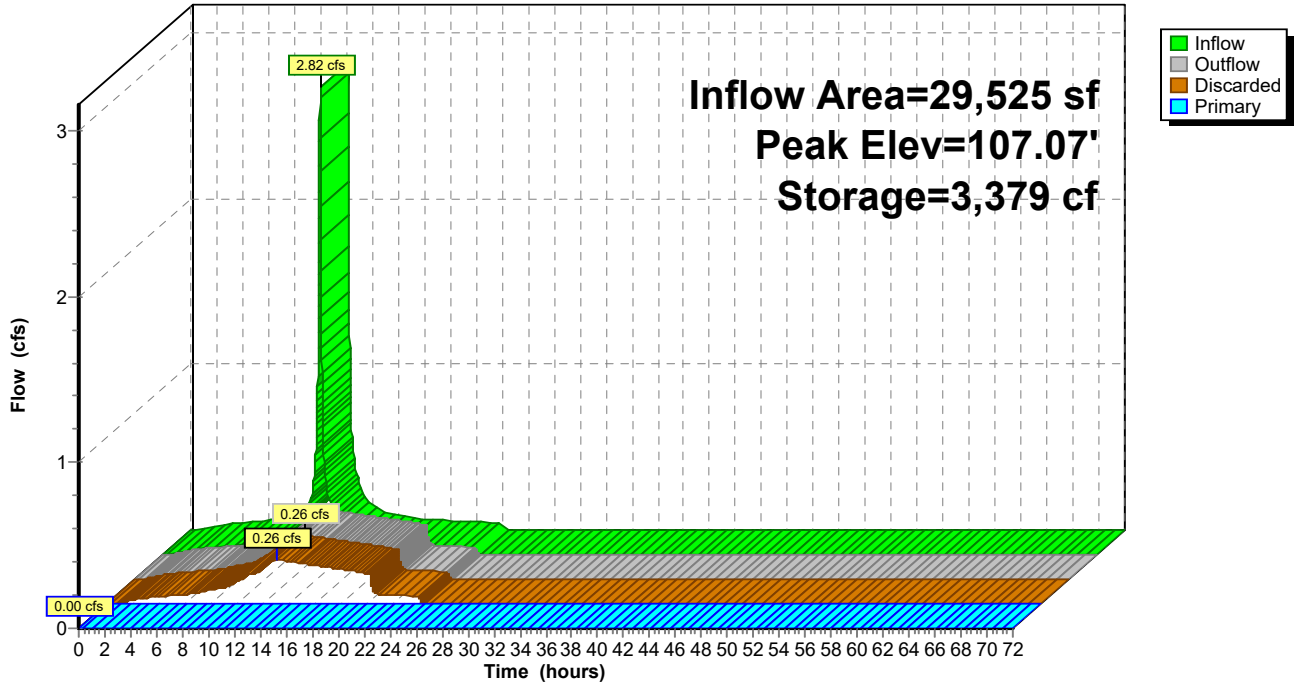
Device	Routing	Invert	Outlet Devices
#1	Primary	104.00'	12.0" Round Culvert L= 45.0' Ke= 0.500 Inlet / Outlet Invert= 104.00' / 103.50' S= 0.0111 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	107.10'	6.0" W x 4.0" H Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	108.20'	6.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#4	Discarded	105.00'	3.150 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 101.00' Phase-In= 0.10'

Discarded OutFlow Max=0.26 cfs @ 13.08 hrs HW=107.07' (Free Discharge)
 ↳4=Exfiltration (Controls 0.26 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=105.00' TW=0.00' (Dynamic Tailwater)
 ↳1=Culvert (Passes 0.00 cfs of 2.67 cfs potential flow)
 ↳2=Orifice/Grate (Controls 0.00 cfs)
 ↳3=Broad-Crested Rectangular Weir(Controls 0.00 cfs)

Pond B-1: StormTech SC-800 Subsurface Infiltration System

Hydrograph



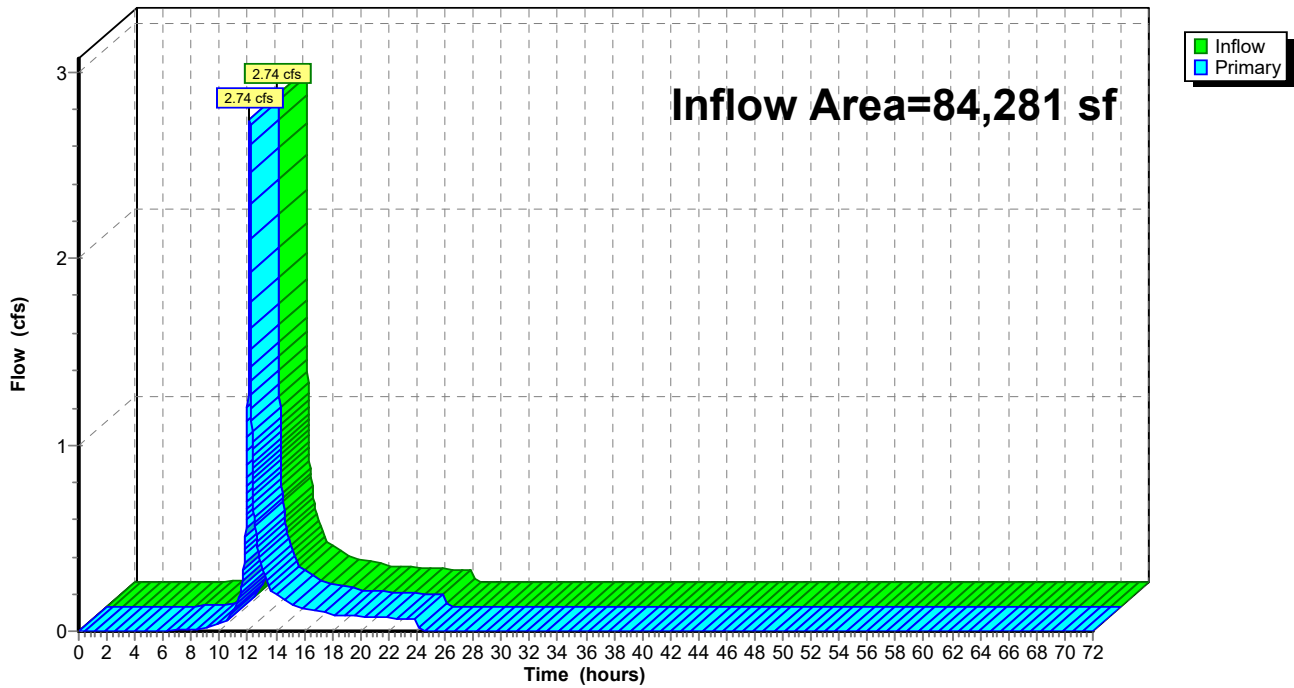
Summary for Link POI-1: Wetlands

Inflow Area = 84,281 sf, 36.81% Impervious, Inflow Depth = 1.32" for 10-Year event
Inflow = 2.74 cfs @ 12.14 hrs, Volume= 9,249 cf
Primary = 2.74 cfs @ 12.14 hrs, Volume= 9,249 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link POI-1: Wetlands

Hydrograph



Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentEX-1: Runoff to Wetlands Runoff Area=84,280 sf 13.70% Impervious Runoff Depth=2.95"
Flow Length=204' Tc=11.2 min CN=68/98 Runoff=4.98 cfs 20,702 cf

SubcatchmentP-1A: Direct to Wetlands Runoff Area=54,756 sf 3.83% Impervious Runoff Depth=2.99"
Flow Length=238' Tc=6.6 min CN=72/98 Runoff=4.07 cfs 13,654 cf

SubcatchmentP-1B: Parking Lot Runoff Area=29,525 sf 97.99% Impervious Runoff Depth=5.57"
Tc=6.0 min CN=66/98 Runoff=3.57 cfs 13,695 cf

Pond B-1: StormTech SC-800 Subsurface Peak Elev=107.49' Storage=4,056 cf Inflow=3.57 cfs 13,695 cf
Discarded=0.28 cfs 12,534 cf Primary=0.37 cfs 1,161 cf Outflow=0.65 cfs 13,695 cf

Link POI-1: Wetlands Inflow=4.07 cfs 14,814 cf
Primary=4.07 cfs 14,814 cf

Total Runoff Area = 168,561 sf Runoff Volume = 48,051 cf Average Runoff Depth = 3.42"
74.74% Pervious = 125,986 sf 25.26% Impervious = 42,575 sf

Summary for Subcatchment EX-1: Runoff to Wetlands

Runoff = 4.98 cfs @ 12.19 hrs, Volume= 20,702 cf, Depth= 2.95"

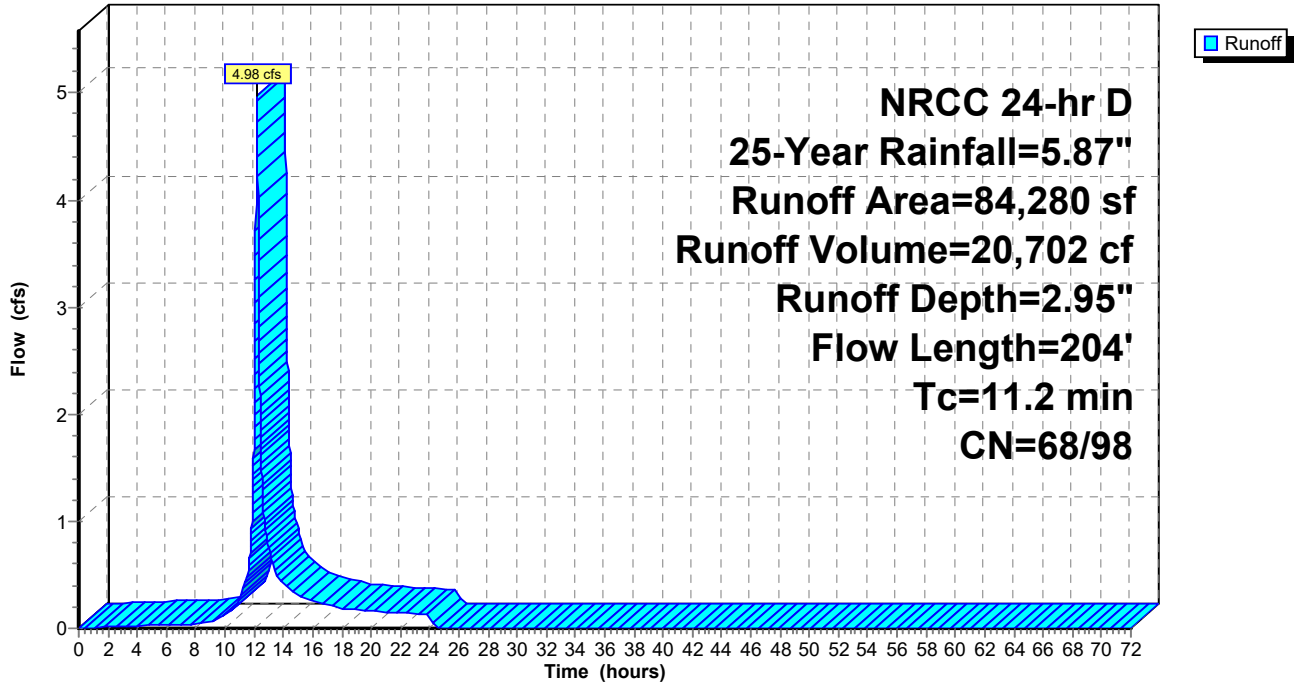
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
NRCC 24-hr D 25-Year Rainfall=5.87"

Area (sf)	CN	Description
11,547	98	Unconnected pavement, HSG D
7,989	30	Woods, Good, HSG A
44,941	77	Woods, Good, HSG D
5,755	80	>75% Grass cover, Good, HSG D
7,174	74	>75% Grass cover, Good, HSG C
6,874	39	>75% Grass cover, Good, HSG A
84,280	72	Weighted Average
72,733	68	86.30% Pervious Area
11,547	98	13.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.3	96	0.1200	0.15		Sheet Flow, 1A-1B Woods: Light underbrush n= 0.400 P2= 3.09"
0.6	63	0.0670	1.81		Shallow Concentrated Flow, 1B-1C Short Grass Pasture Kv= 7.0 fps
0.3	45	0.2000	2.24		Shallow Concentrated Flow, 1C-1D Woodland Kv= 5.0 fps
11.2	204	Total			

Subcatchment EX-1: Runoff to Wetlands

Hydrograph



Summary for Subcatchment P-1A: Direct to Wetlands

Runoff = 4.07 cfs @ 12.14 hrs, Volume= 13,654 cf, Depth= 2.99"
 Routed to Link POI-1 : Wetlands

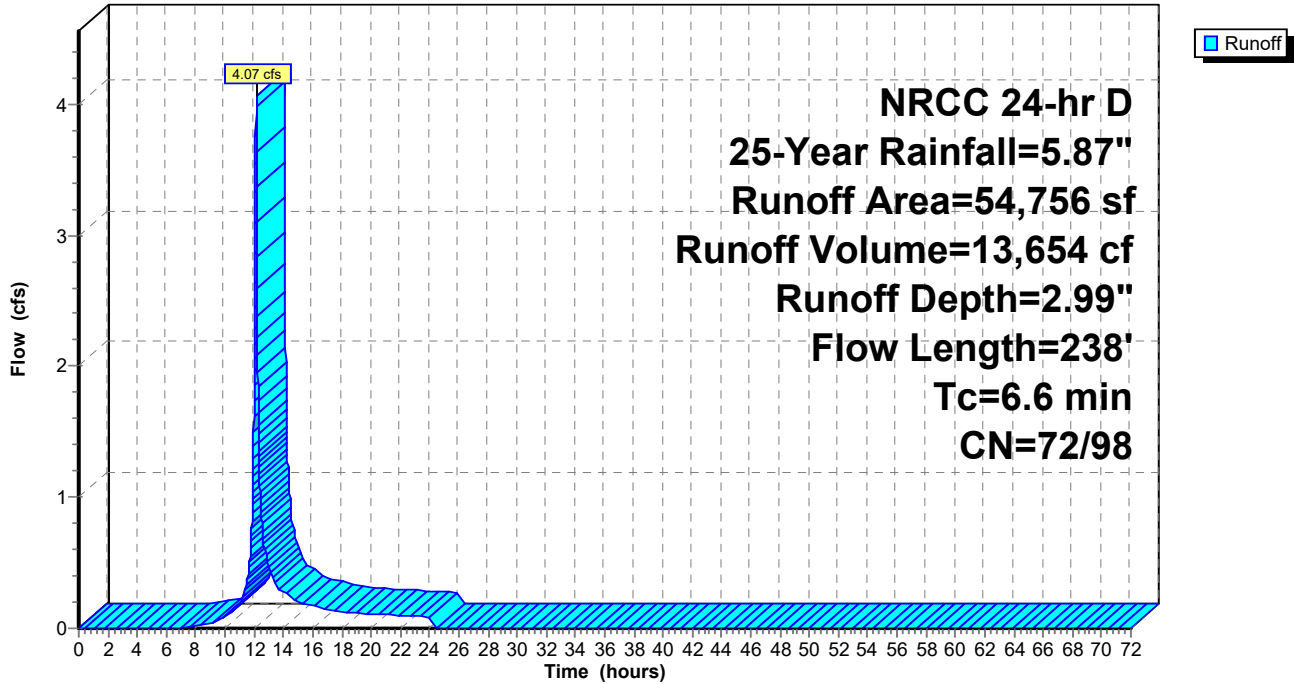
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 NRCC 24-hr D 25-Year Rainfall=5.87"

Area (sf)	CN	Description
* 2,095	98	Impervious
* 582	39	Turf Area, HSG A
* 8,718	80	Turf Area, HSG D
1,088	30	Woods, Good, HSG A
21,265	77	Woods, Good, HSG D
6,416	39	>75% Grass cover, Good, HSG A
674	74	>75% Grass cover, Good, HSG C
13,918	80	>75% Grass cover, Good, HSG D
54,756	73	Weighted Average
52,661	72	96.17% Pervious Area
2,095	98	3.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	100	0.0850	0.30		Sheet Flow, 1A-1B Grass: Short n= 0.150 P2= 3.09"
0.9	105	0.0140	1.90		Shallow Concentrated Flow, 1B-1C Unpaved Kv= 16.1 fps
0.1	33	0.0600	3.94		Shallow Concentrated Flow, 1C-1D Unpaved Kv= 16.1 fps
6.6	238	Total			

Subcatchment P-1A: Direct to Wetlands

Hydrograph



Summary for Subcatchment P-1B: Parking Lot

Runoff = 3.57 cfs @ 12.13 hrs, Volume= 13,695 cf, Depth= 5.57"

Routed to Pond B-1 : StormTech SC-800 Subsurface Infiltration System

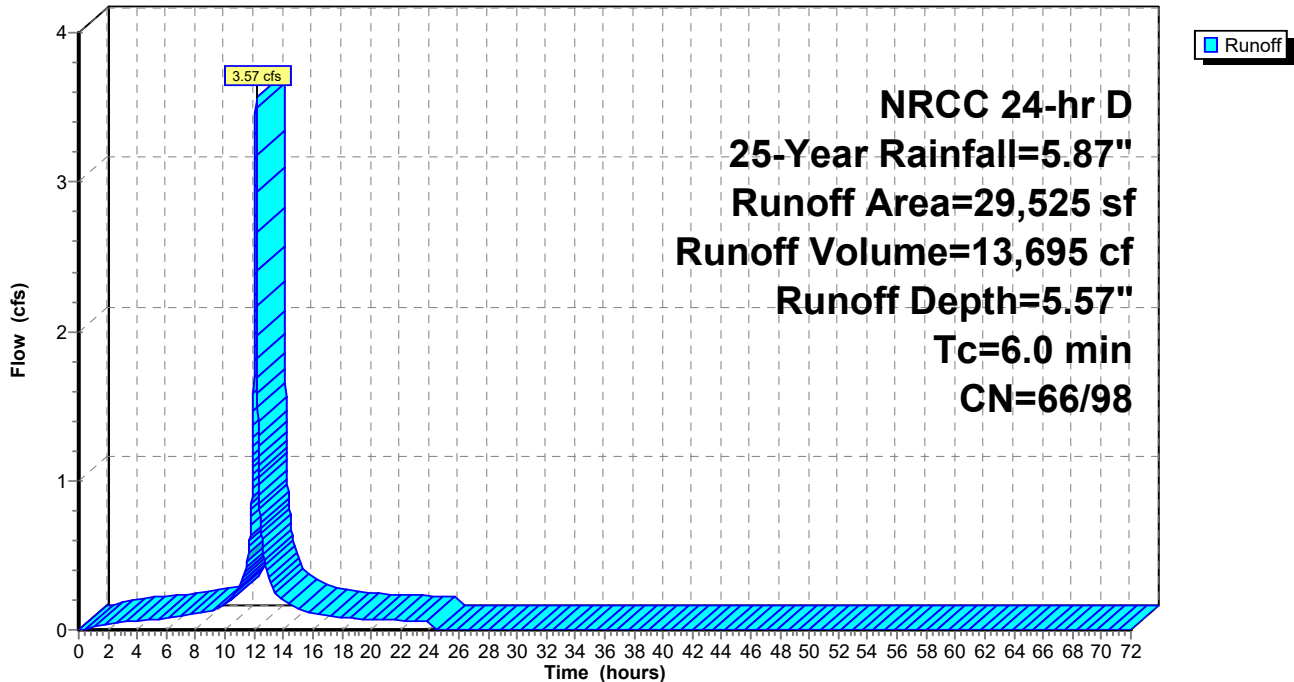
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
NRCC 24-hr D 25-Year Rainfall=5.87"

	Area (sf)	CN	Description
*	28,933	98	Impervious
	238	80	>75% Grass cover, Good, HSG D
	181	74	>75% Grass cover, Good, HSG C
	173	39	>75% Grass cover, Good, HSG A
	29,525	97	Weighted Average
	592	66	2.01% Pervious Area
	28,933	98	97.99% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min

Subcatchment P-1B: Parking Lot

Hydrograph



Summary for Pond B-1: StormTech SC-800 Subsurface Infiltration System

Inflow Area = 29,525 sf, 97.99% Impervious, Inflow Depth = 5.57" for 25-Year event
 Inflow = 3.57 cfs @ 12.13 hrs, Volume= 13,695 cf
 Outflow = 0.65 cfs @ 12.47 hrs, Volume= 13,695 cf, Atten= 82%, Lag= 20.3 min
 Discarded = 0.28 cfs @ 12.47 hrs, Volume= 12,534 cf
 Primary = 0.37 cfs @ 12.47 hrs, Volume= 1,161 cf
 Routed to Link POI-1 : Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 Peak Elev= 107.49' @ 12.47 hrs Surf.Area= 2,357 sf Storage= 4,056 cf

Plug-Flow detention time= 98.5 min calculated for 13,693 cf (100% of inflow)
 Center-of-Mass det. time= 98.5 min (847.2 - 748.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	105.00'	2,241 cf	34.75"W x 67.82'L x 3.75'H Field A 8,837 cf Overall - 3,235 cf Embedded = 5,602 cf x 40.0% Voids
#2A	105.50'	3,235 cf	ADS_StormTech SC-800 +Cap x 63 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 63 Chambers in 7 Rows Cap Storage= 3.4 cf x 2 x 7 rows = 47.9 cf
		5,476 cf	Total Available Storage

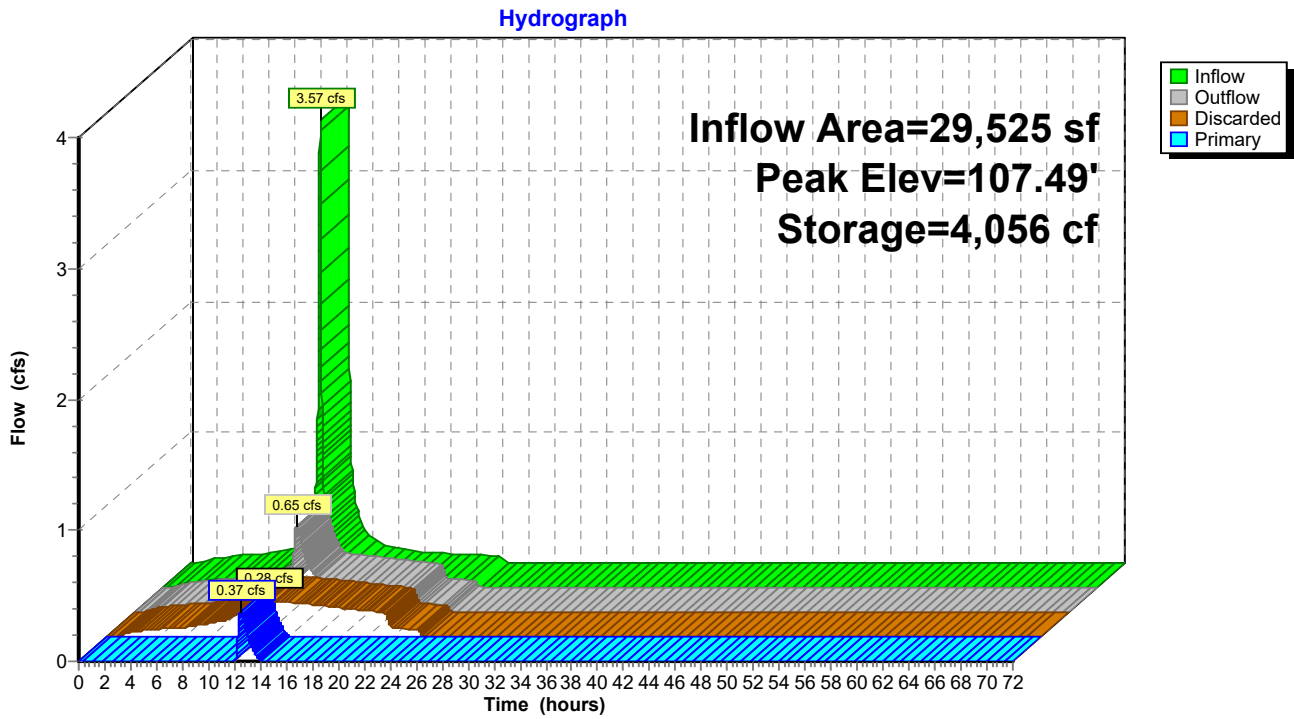
Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	104.00'	12.0" Round Culvert L= 45.0' Ke= 0.500 Inlet / Outlet Invert= 104.00' / 103.50' S= 0.0111 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	107.10'	6.0" W x 4.0" H Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	108.20'	6.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#4	Discarded	105.00'	3.150 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 101.00' Phase-In= 0.10'

Discarded OutFlow Max=0.28 cfs @ 12.47 hrs HW=107.49' (Free Discharge)
 ↑4=Exfiltration (Controls 0.28 cfs)

Primary OutFlow Max=0.37 cfs @ 12.47 hrs HW=107.49' TW=0.00' (Dynamic Tailwater)
 ↑1=Culvert (Passes 0.37 cfs of 6.39 cfs potential flow)
 ↑2=Orifice/Grate (Orifice Controls 0.37 cfs @ 2.23 fps)
 ↑3=Broad-Crested Rectangular Weir(Controls 0.00 cfs)

Pond B-1: StormTech SC-800 Subsurface Infiltration System



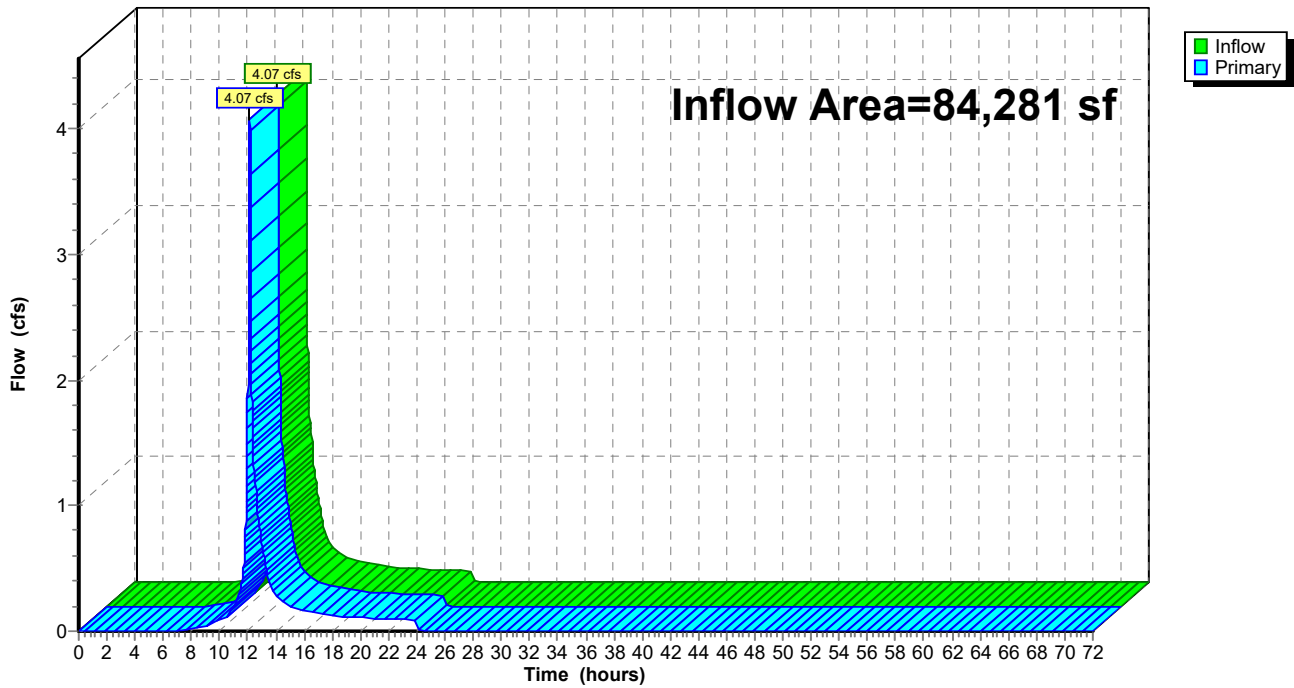
Summary for Link POI-1: Wetlands

Inflow Area = 84,281 sf, 36.81% Impervious, Inflow Depth = 2.11" for 25-Year event
Inflow = 4.07 cfs @ 12.14 hrs, Volume= 14,814 cf
Primary = 4.07 cfs @ 12.14 hrs, Volume= 14,814 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link POI-1: Wetlands

Hydrograph



Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentEX-1: Runoff to Wetlands Runoff Area=84,280 sf 13.70% Impervious Runoff Depth=5.03"
Flow Length=204' Tc=11.2 min CN=68/98 Runoff=8.59 cfs 35,327 cf

SubcatchmentP-1A: Direct to Wetlands Runoff Area=54,756 sf 3.83% Impervious Runoff Depth=5.13"
Flow Length=238' Tc=6.6 min CN=72/98 Runoff=6.94 cfs 23,411 cf

SubcatchmentP-1B: Parking Lot Runoff Area=29,525 sf 97.99% Impervious Runoff Depth=8.04"
Tc=6.0 min CN=66/98 Runoff=5.11 cfs 19,790 cf

Pond B-1: StormTech SC-800 Subsurface Peak Elev=108.45' Storage=5,189 cf Inflow=5.11 cfs 19,790 cf
Discarded=0.32 cfs 15,412 cf Primary=2.93 cfs 4,378 cf Outflow=3.25 cfs 19,790 cf

Link POI-1: Wetlands Inflow=8.33 cfs 27,789 cf
Primary=8.33 cfs 27,789 cf

Total Runoff Area = 168,561 sf Runoff Volume = 78,529 cf Average Runoff Depth = 5.59"
74.74% Pervious = 125,986 sf 25.26% Impervious = 42,575 sf

Summary for Subcatchment EX-1: Runoff to Wetlands

Runoff = 8.59 cfs @ 12.19 hrs, Volume= 35,327 cf, Depth= 5.03"

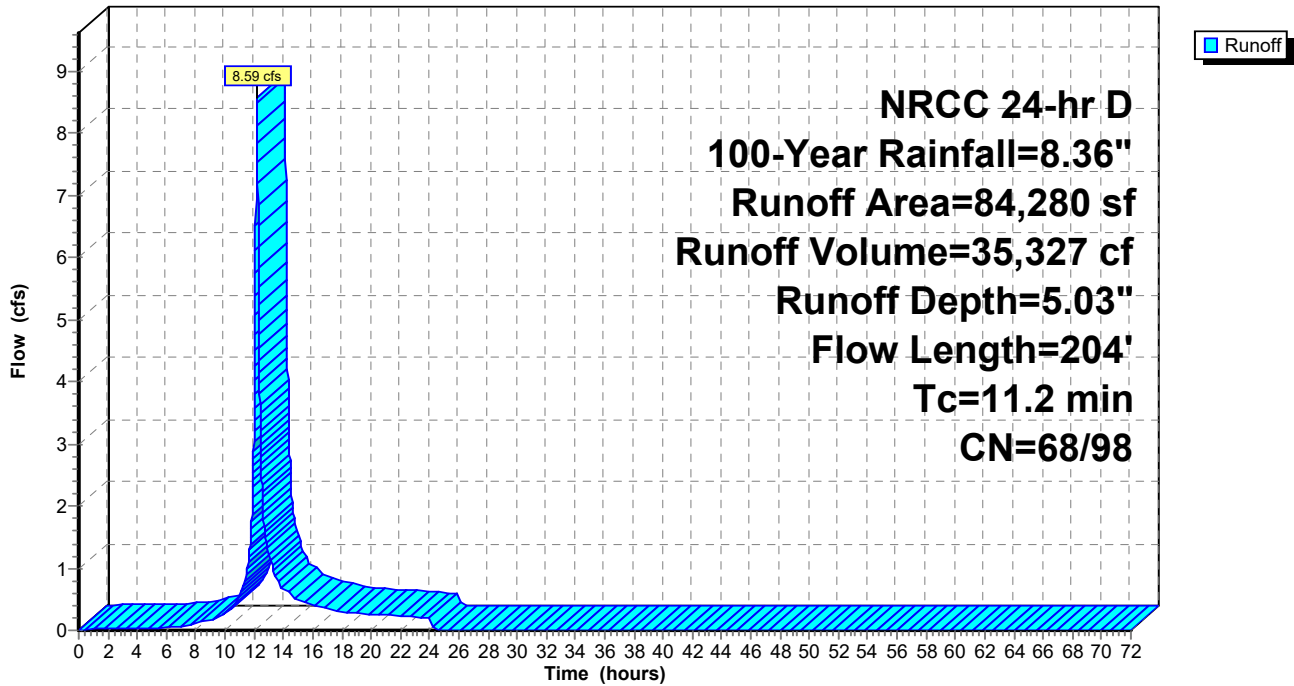
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
NRCC 24-hr D 100-Year Rainfall=8.36"

Area (sf)	CN	Description
11,547	98	Unconnected pavement, HSG D
7,989	30	Woods, Good, HSG A
44,941	77	Woods, Good, HSG D
5,755	80	>75% Grass cover, Good, HSG D
7,174	74	>75% Grass cover, Good, HSG C
6,874	39	>75% Grass cover, Good, HSG A
84,280	72	Weighted Average
72,733	68	86.30% Pervious Area
11,547	98	13.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.3	96	0.1200	0.15		Sheet Flow, 1A-1B Woods: Light underbrush n= 0.400 P2= 3.09"
0.6	63	0.0670	1.81		Shallow Concentrated Flow, 1B-1C Short Grass Pasture Kv= 7.0 fps
0.3	45	0.2000	2.24		Shallow Concentrated Flow, 1C-1D Woodland Kv= 5.0 fps
11.2	204	Total			

Subcatchment EX-1: Runoff to Wetlands

Hydrograph



Summary for Subcatchment P-1A: Direct to Wetlands

Runoff = 6.94 cfs @ 12.14 hrs, Volume= 23,411 cf, Depth= 5.13"
 Routed to Link POI-1 : Wetlands

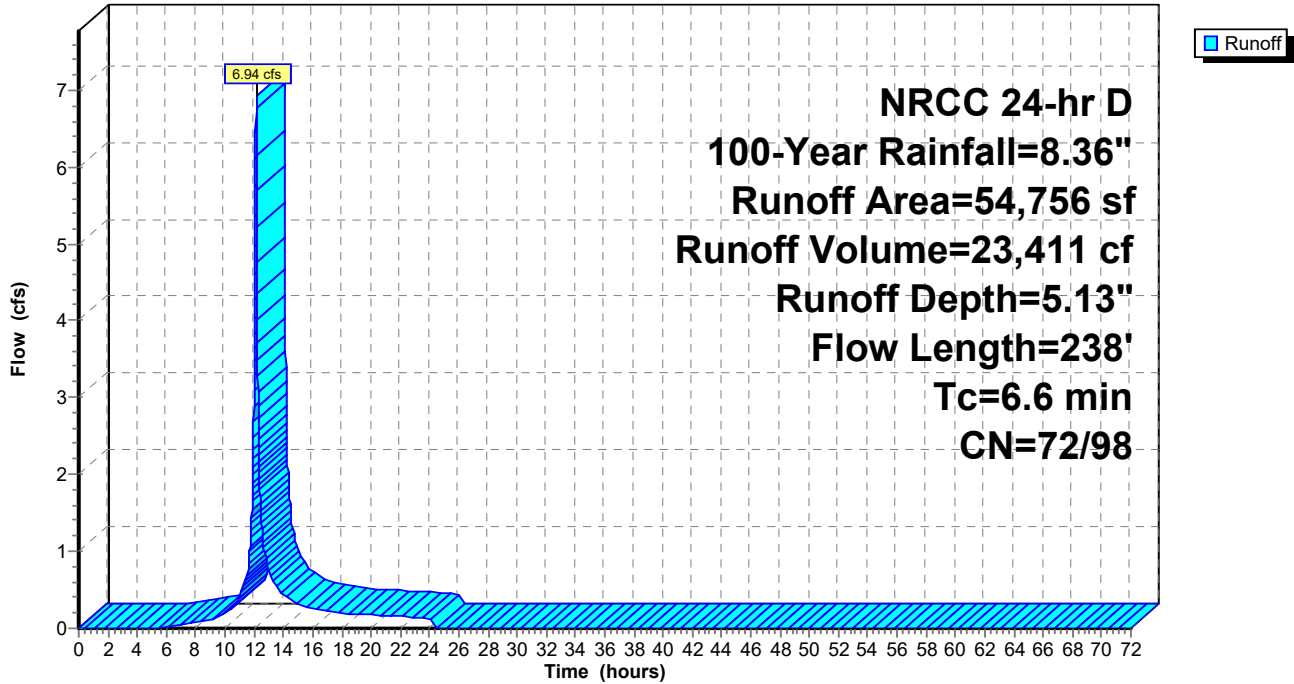
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 NRCC 24-hr D 100-Year Rainfall=8.36"

Area (sf)	CN	Description
* 2,095	98	Impervious
* 582	39	Turf Area, HSG A
* 8,718	80	Turf Area, HSG D
1,088	30	Woods, Good, HSG A
21,265	77	Woods, Good, HSG D
6,416	39	>75% Grass cover, Good, HSG A
674	74	>75% Grass cover, Good, HSG C
13,918	80	>75% Grass cover, Good, HSG D
54,756	73	Weighted Average
52,661	72	96.17% Pervious Area
2,095	98	3.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	100	0.0850	0.30		Sheet Flow, 1A-1B Grass: Short n= 0.150 P2= 3.09"
0.9	105	0.0140	1.90		Shallow Concentrated Flow, 1B-1C Unpaved Kv= 16.1 fps
0.1	33	0.0600	3.94		Shallow Concentrated Flow, 1C-1D Unpaved Kv= 16.1 fps
6.6	238	Total			

Subcatchment P-1A: Direct to Wetlands

Hydrograph



Summary for Subcatchment P-1B: Parking Lot

Runoff = 5.11 cfs @ 12.13 hrs, Volume= 19,790 cf, Depth= 8.04"

Routed to Pond B-1 : StormTech SC-800 Subsurface Infiltration System

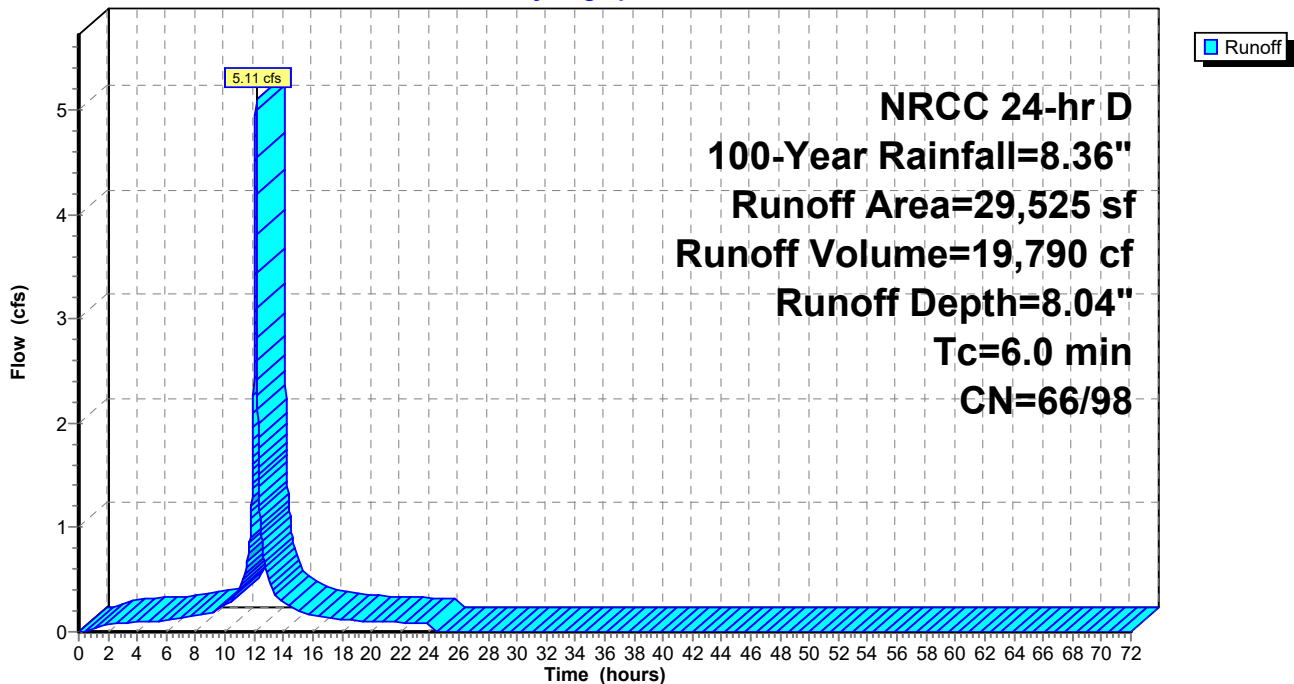
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
NRCC 24-hr D 100-Year Rainfall=8.36"

	Area (sf)	CN	Description
*	28,933	98	Impervious
	238	80	>75% Grass cover, Good, HSG D
	181	74	>75% Grass cover, Good, HSG C
	173	39	>75% Grass cover, Good, HSG A
	29,525	97	Weighted Average
	592	66	2.01% Pervious Area
	28,933	98	97.99% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min

Subcatchment P-1B: Parking Lot

Hydrograph



Summary for Pond B-1: StormTech SC-800 Subsurface Infiltration System

Inflow Area = 29,525 sf, 97.99% Impervious, Inflow Depth = 8.04" for 100-Year event
 Inflow = 5.11 cfs @ 12.13 hrs, Volume= 19,790 cf
 Outflow = 3.25 cfs @ 12.20 hrs, Volume= 19,790 cf, Atten= 36%, Lag= 4.1 min
 Discarded = 0.32 cfs @ 12.20 hrs, Volume= 15,412 cf
 Primary = 2.93 cfs @ 12.20 hrs, Volume= 4,378 cf
 Routed to Link POI-1 : Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 Peak Elev= 108.45' @ 12.20 hrs Surf.Area= 2,357 sf Storage= 5,189 cf

Plug-Flow detention time= 93.0 min calculated for 19,788 cf (100% of inflow)
 Center-of-Mass det. time= 93.0 min (836.3 - 743.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	105.00'	2,241 cf	34.75"W x 67.82'L x 3.75'H Field A 8,837 cf Overall - 3,235 cf Embedded = 5,602 cf x 40.0% Voids
#2A	105.50'	3,235 cf	ADS_StormTech SC-800 +Cap x 63 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 63 Chambers in 7 Rows Cap Storage= 3.4 cf x 2 x 7 rows = 47.9 cf
		5,476 cf	Total Available Storage

Storage Group A created with Chamber Wizard

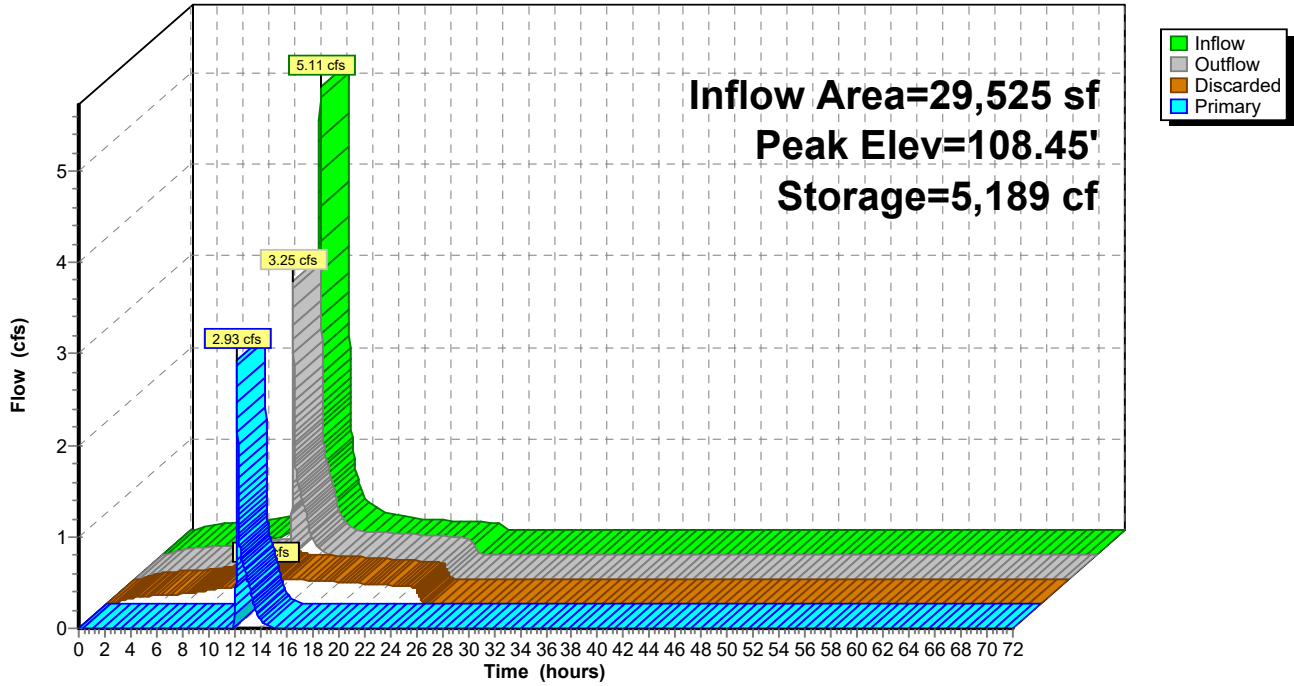
Device	Routing	Invert	Outlet Devices
#1	Primary	104.00'	12.0" Round Culvert L= 45.0' Ke= 0.500 Inlet / Outlet Invert= 104.00' / 103.50' S= 0.0111 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	107.10'	6.0" W x 4.0" H Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	108.20'	6.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#4	Discarded	105.00'	3.150 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 101.00' Phase-In= 0.10'

Discarded OutFlow Max=0.32 cfs @ 12.20 hrs HW=108.44' (Free Discharge)
 ↑4=Exfiltration (Controls 0.32 cfs)

Primary OutFlow Max=2.91 cfs @ 12.20 hrs HW=108.44' TW=0.00' (Dynamic Tailwater)
 ↑1=Culvert (Passes 2.91 cfs of 7.33 cfs potential flow)
 ↑2=Orifice/Grate (Orifice Controls 0.87 cfs @ 5.22 fps)
 ↑3=Broad-Crested Rectangular Weir(Weir Controls 2.04 cfs @ 1.40 fps)

Pond B-1: StormTech SC-800 Subsurface Infiltration System

Hydrograph



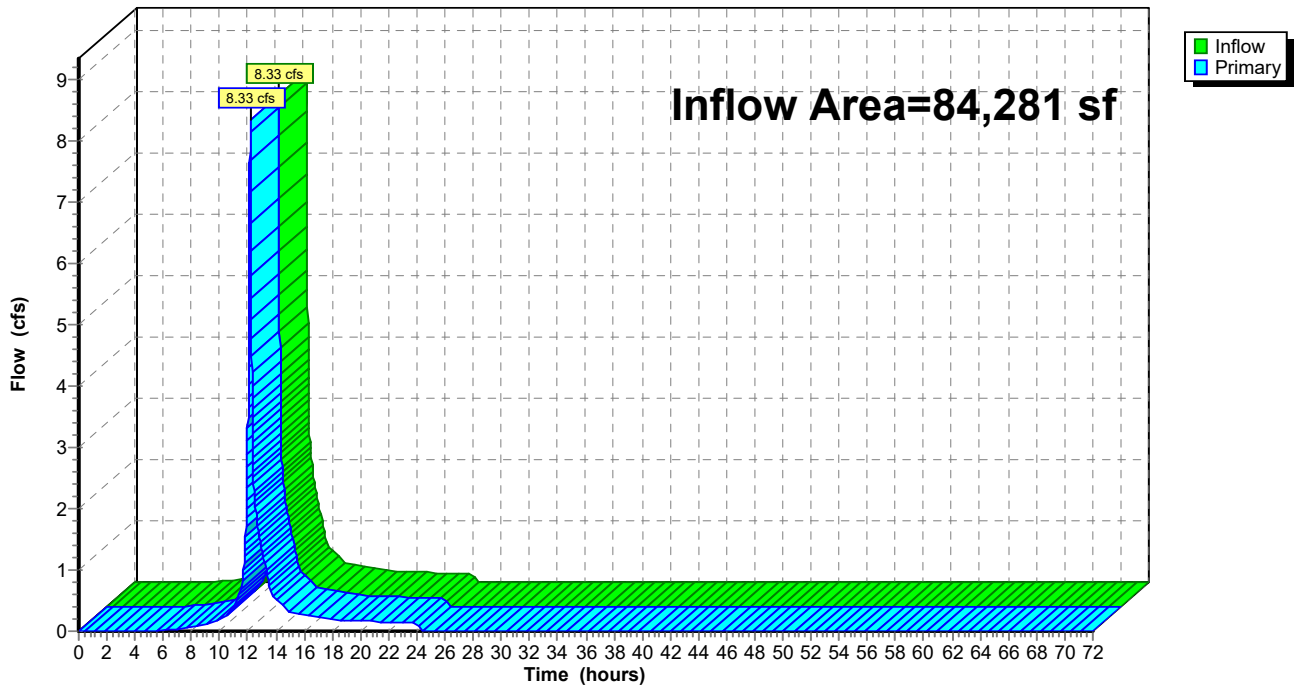
Summary for Link POI-1: Wetlands

Inflow Area = 84,281 sf, 36.81% Impervious, Inflow Depth = 3.96" for 100-Year event
Inflow = 8.33 cfs @ 12.18 hrs, Volume= 27,789 cf
Primary = 8.33 cfs @ 12.18 hrs, Volume= 27,789 cf, Atten= 0%, Lag= 0.0 min

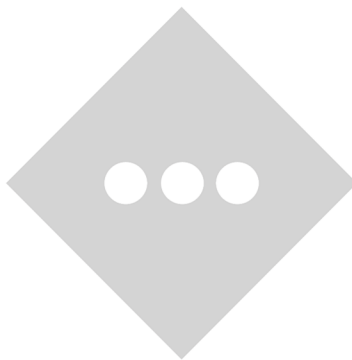
Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link POI-1: Wetlands

Hydrograph



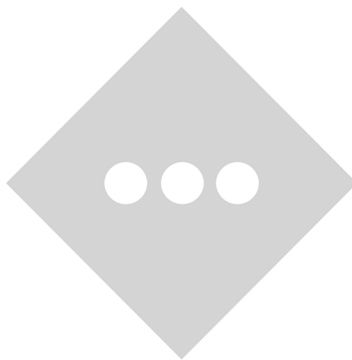
APPENDIX C-4
INFILTRATION BASIN STAGE-STORAGE
TABLES



Stage-Area-Storage for Pond B-1: StormTech SC-800 Subsurface Infiltration System

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
105.00	2,357	0
105.10	2,357	94
105.20	2,357	189
105.30	2,357	283
105.40	2,357	377
105.50	2,357	471
105.60	2,357	668
105.70	2,357	863
105.80	2,357	1,057
105.90	2,357	1,250
106.00	2,357	1,441
106.10	2,357	1,631
106.20	2,357	1,820
106.30	2,357	2,007
106.40	2,357	2,192
106.50	2,357	2,375
106.60	2,357	2,556
106.70	2,357	2,735
106.80	2,357	2,912
106.90	2,357	3,086
107.00	2,357	3,258
107.10	2,357	3,426
107.20	2,357	3,592
107.30	2,357	3,754
107.40	2,357	3,912
107.50	2,357	4,066
107.60	2,357	4,216
107.70	2,357	4,360
107.80	2,357	4,498
107.90	2,357	4,628
108.00	2,357	4,747
108.10	2,357	4,855
108.20	2,357	4,956
108.30	2,357	5,052
108.40	2,357	5,146
108.50	2,357	5,240
108.60	2,357	5,335
108.70	2,357	5,429

APPENDIX C-5
INFILTRATION BASIN STAGE-DISCHARGE
TABLES



Stage-Discharge for Pond B-1: StormTech SC-800 Subsurface Infiltration System

Elevation (feet)	Discharge (cfs)	Discarded (cfs)	Primary (cfs)
105.00	0.00	0.00	0.00
105.10	0.18	0.18	0.00
105.20	0.18	0.18	0.00
105.30	0.18	0.18	0.00
105.40	0.19	0.19	0.00
105.50	0.19	0.19	0.00
105.60	0.20	0.20	0.00
105.70	0.20	0.20	0.00
105.80	0.21	0.21	0.00
105.90	0.21	0.21	0.00
106.00	0.21	0.21	0.00
106.10	0.22	0.22	0.00
106.20	0.22	0.22	0.00
106.30	0.23	0.23	0.00
106.40	0.23	0.23	0.00
106.50	0.24	0.24	0.00
106.60	0.24	0.24	0.00
106.70	0.24	0.24	0.00
106.80	0.25	0.25	0.00
106.90	0.25	0.25	0.00
107.00	0.26	0.26	0.00
107.10	0.26	0.26	0.00
107.20	0.32	0.27	0.05
107.30	0.41	0.27	0.14
107.40	0.54	0.27	0.26
107.50	0.66	0.28	0.38
107.60	0.74	0.28	0.46
107.70	0.81	0.29	0.52
107.80	0.88	0.29	0.58
107.90	0.93	0.30	0.64
108.00	0.99	0.30	0.69
108.10	1.04	0.31	0.73
108.20	1.08	0.31	0.77
108.30	1.66	0.31	1.35
108.40	2.67	0.32	2.36
108.50	4.03	0.32	3.71
108.60	5.68	0.33	5.36
108.70	7.65	0.33	7.32

APPENDIX D

SITE PLAN SHEETS

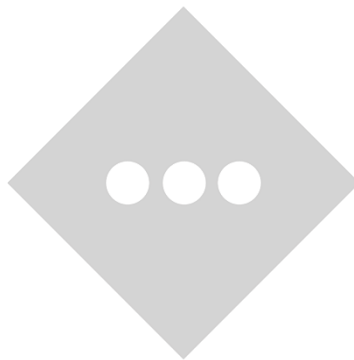
INVENTORY

FIGURE 1: SITE PLAN

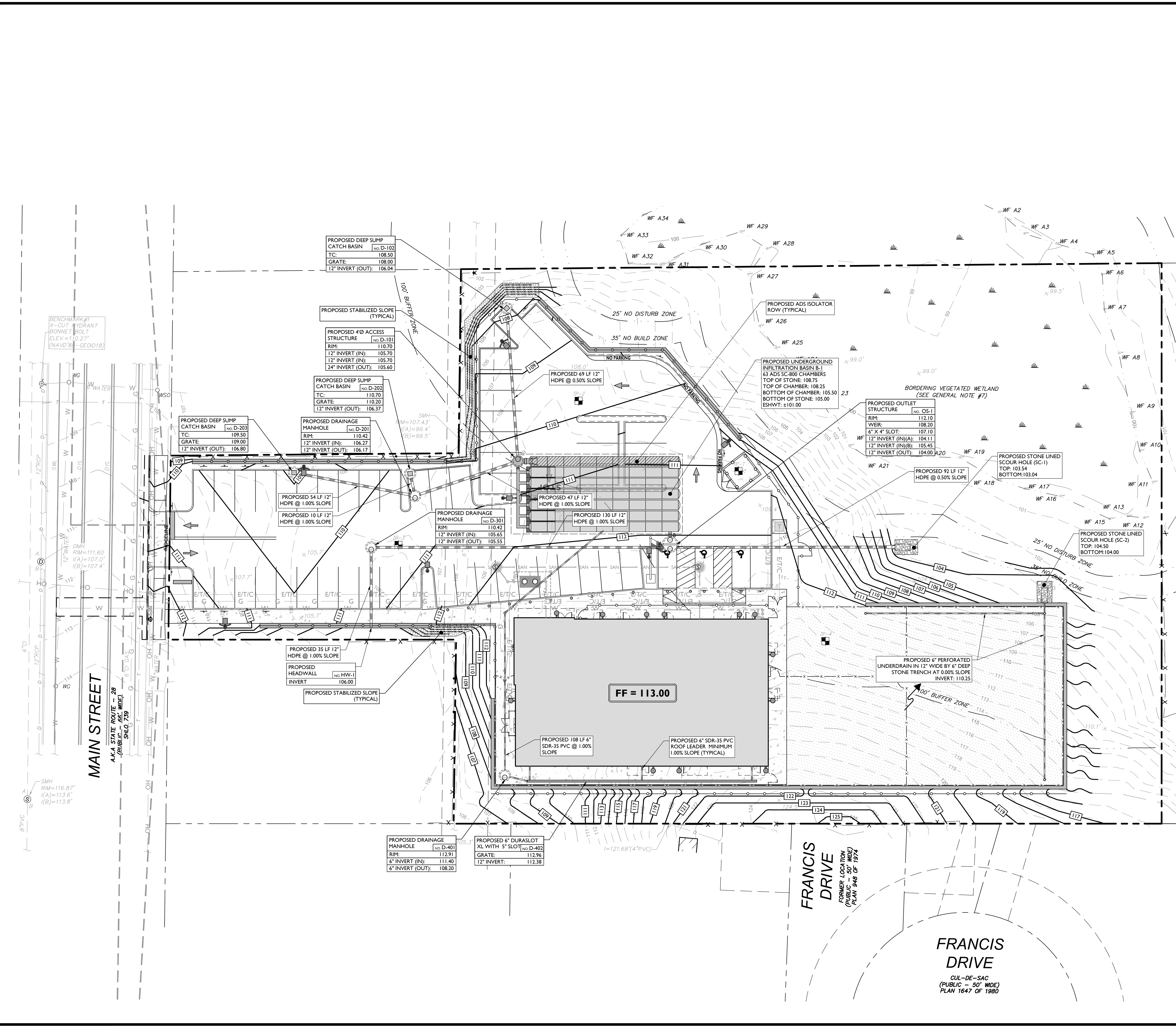
FIGURE 2: STORMWATER MANAGEMENT PLAN

FIGURE 3: LANDSCAPING PLAN

FIGURE 4: SOIL EROSION & SEDIMENT CONTROL PLAN



2:10/20/2018 09:52:40 28113 PRIMROSE SCHOOLS - 88 MAIN STREET, FRAMINGHAM, MA 01901
 2:10/20/2018 09:52:40 28113 PRIMROSE SCHOOLS - 88 MAIN STREET, FRAMINGHAM, MA 01901



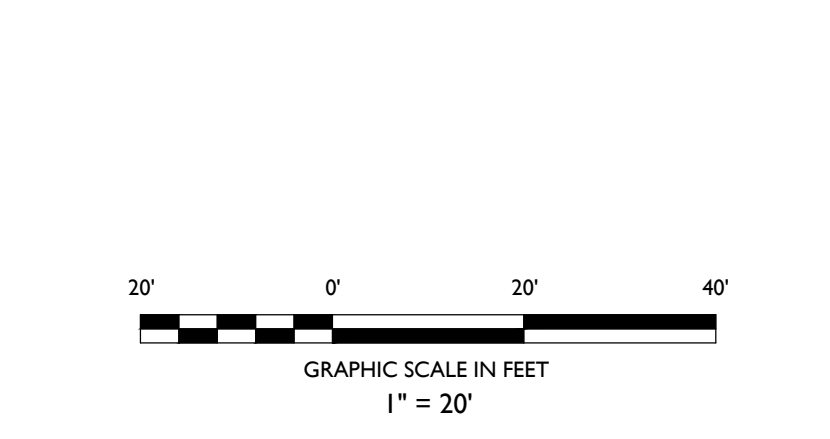
SYMBOL	DESCRIPTION
---	PROPERTY LINE
100	PROPOSED GRADING CONTOUR
---	PROPOSED GRADING RIDGELINE
○	PROPOSED STORMWATER STRUCTURES
---	PROPOSED TRENCH DRAIN
---	PROPOSED STORMWATER PIPING
○	PROPOSED UNDERGROUND OUTLET STRUCTURE

- DRAINAGE AND UTILITY NOTES**
- THE CONTRACTOR TO PERFORM A TEST PIT PRIOR TO CONSTRUCTION (RECOMMEND 30 DAYS PRIOR) AT LOCATIONS OF EXISTING UTILITY CROSSINGS FOR STORMWATER IMPROVEMENTS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY STONEFIELD ENGINEERING & DESIGN, LLC IN WRITING.
 - CONTRACTOR SHALL START CONSTRUCTION OF STORM LINES AT THE LOWEST INVERT AND WORK UP GRADIENT.
 - THE CONTRACTOR IS REQUIRED TO CALL THE APPROPRIATE AUTHORITY FOR NOTICE OF CONSTRUCTION/EXCAVATION AND UTILITY MARK OUT PRIOR TO THE START OF CONSTRUCTION IN ACCORDANCE WITH STATE LAW. CONTRACTOR IS REQUIRED TO CONFIRM THE HORIZONTAL AND VERTICAL LOCATION OF UTILITIES IN THE FIELD. SHOULD A DISCREPANCY EXIST BETWEEN THE FIELD LOCATION OF A UTILITY AND THE LOCATION SHOWN ON THE PLAN SET OR SURVEY, THE CONTRACTOR SHALL NOTIFY STONEFIELD ENGINEERING & DESIGN, LLC IMMEDIATELY IN WRITING.
 - THE CONTRACTOR IS RESPONSIBLE TO MAINTAIN A RECORD OF THE AS-BUILT LOCATIONS OF ALL PROPOSED UNDERGROUND INFRASTRUCTURE. THE CONTRACTOR SHALL NOTE ANY DISCREPANCIES BETWEEN THE AS-BUILT LOCATIONS AND THE LOCATIONS DEPICTED WITHIN THE PLAN SET. THIS RECORD SHALL BE PROVIDED TO THE OWNER FOLLOWING COMPLETION OF WORK.

- EXCAVATION, SOIL PREPARATION, AND DEWATERING NOTES**
- THE CONTRACTOR IS REQUIRED TO REVIEW THE REFERENCED GEOTECHNICAL DOCUMENTS PRIOR TO CONSTRUCTION. THESE DOCUMENTS SHALL BE CONSIDERED A PART OF THE PLAN SET.
 - THE CONTRACTOR IS REQUIRED TO PREPARE SUBGRADE SOILS BENEATH ALL PROPOSED IMPROVEMENTS AND BACKFILL ALL EXCAVATIONS IN ACCORDANCE WITH RECOMMENDATIONS BY THE GEOTECHNICAL ENGINEER OF RECORD.
 - THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING SHORING FOR ALL EXCAVATIONS AS REQUIRED. CONTRACTOR SHALL HAVE THE SHORING DESIGN PREPARED BY A QUALIFIED PROFESSIONAL SHORING DESIGNER. SUCH DESIGN SHALL BE SUBMITTED TO STONEFIELD ENGINEERING & DESIGN, LLC AND THE OWNER PRIOR TO THE START OF CONSTRUCTION.
 - THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT ALL OPEN EXCAVATIONS ARE PROTECTED IN ACCORDANCE WITH THE LATEST OSHA REGULATIONS.
 - THE CONTRACTOR IS RESPONSIBLE FOR ANY DEWATERING DESIGN AND OPERATIONS, AS REQUIRED, TO CONSTRUCT THE PROPOSED IMPROVEMENTS. THE CONTRACTOR SHALL OBTAIN ANY REQUIRED PERMITS FOR DEWATERING OPERATIONS AND GROUNDWATER DISPOSAL.

- STORMWATER INFILTRATION BMP CONSTRUCTION NOTES**
- PRIOR TO THE START OF CONSTRUCTION, ANY AREA DESIGNATED TO BE USED FOR AN INFILTRATION BMP (E.G. BASIN, BIOTENTION AREA, ETC.) SHALL BE FENCED OFF AND SHALL NOT BE UTILIZED AS STORAGE FOR CONSTRUCTION EQUIPMENT OR AS A STOCKPILE AREA FOR CONSTRUCTION MATERIALS. NO ACTIVITY SHALL BE PERMITTED WITHIN THE INFILTRATION BASIN AREA UNLESS RELATED TO THE CONSTRUCTION OF THE INFILTRATION BASIN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY ALL SUBCONTRACTORS OF BASIN AREA RESTRICTIONS.
 - THE CONTRACTOR SHALL MAKE EVERY EFFORT, WHERE PRACTICAL, TO AVOID SUBGRADE SOIL COMPACTION IN THE AREAS DESIGNATED TO BE USED FOR AN INFILTRATION BMP.
 - ALL EXCAVATION WITHIN THE LIMITS OF ANY INFILTRATION BMP SHALL BE PERFORMED WITH THE LIGHTEST PRACTICAL EXCAVATION EQUIPMENT. ALL EXCAVATION EQUIPMENT SHALL BE PLACED OUTSIDE THE LIMITS OF THE BASIN WHERE FEASIBLE. THE USE OF LIGHT-WEIGHT, RUBBER-TIRED EQUIPMENT (LESS THAN 8 PSI APPLIED TO THE GROUND SURFACE) IS RECOMMENDED WITHIN THE BASIN LIMITS.
 - THE SEQUENCE OF SITE CONSTRUCTION SHALL BE COORDINATED WITH BASIN CONSTRUCTION TO ADHERE TO SEQUENCING LIMITATIONS.
 - DURING THE FINAL GRADING OF AN INFILTRATION BASIN, THE BOTTOM OF THE BASIN SHALL BE DEEPLY TILLED WITH A ROTARY TILLER OR DISC HARROW AND THEN SMOOTHED OUT WITH A LEVELING DRAW OR EQUIVALENT GRADING EQUIPMENT. ALL GRADING EQUIPMENT SHALL BE LOCATED OUTSIDE OF THE BASIN BOTTOM WHERE FEASIBLE.
 - THE CONTRACTOR SHALL NOTIFY THE MUNICIPALITY TO DETERMINE IF WITNESS TESTING IS REQUIRED DURING INFILTRATION BASIN EXCAVATION AND/OR SOIL INFILTRATION TESTING.

- STORMWATER UNDERGROUND BMP CONSTRUCTION NOTES**
- THE CONTRACTOR SHALL INSTALL AND BACKFILL THE UNDERGROUND BMP IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS.
 - UNDERGROUND BASINS SHALL UTILIZE A STONE BACKFILL WITH A MINIMUM VOID RATIO OF 40%.
 - NO CONSTRUCTION LOADING OVER UNDERGROUND BASINS IS PERMITTED UNTIL BACKFILL IS COMPLETE PER THE MANUFACTURER'S SPECIFICATIONS. NO VEHICLES SHALL BE STAGED OR OPERATE FROM A FIXED POSITION OVER THE BASIN.



ISSUED FOR TOWN COMMENTS	ISSUED FOR MUNICIPAL SUBMISSION	DATE	BY
01	00	05/06/2025	SCL
		03/07/2025	AJD

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FRANCHISING COMPANY

PROPOSED CHILD DAY CARE FACILITY

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 885 MAIN STREET
 TOWN OF READING
 MIDDLESEX COUNTY, MASSACHUSETTS

JOSHUA H. KLINE, P.E.
 MASSACHUSETTS LICENSE NO. 53936
 LICENSED PROFESSIONAL ENGINEER

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SCALE: 1" = 20' PROJECT ID: BOS-240115

TITLE:
STORMWATER MANAGEMENT PLAN

DRAWING:
C-6

STABILIZATION SPECIFICATIONS:

I.A. TEMPORARY SEEDING AND MULCHING:
GROUND LIMESTONE - APPLIED UNIFORMLY ACCORDING TO SOIL TEST RECOMMENDATIONS.
FERTILIZER - APPLY 1 LBS./1,000 SF OF 10-20-10 OR EQUIVALENT WITH 50% WATER INSOLUBLE NITROGEN (UNLESS A SOIL TEST INDICATES OTHERWISE) WORKED INTO THE SOIL A MINIMUM OF 4".
SEED - PERENNIAL RYEGRASS 100 LBS./ACRE (2.3 LBS./1,000 SF) OR OTHER APPROVED SEEDS; PLANT BETWEEN MARCH 1 AND MAY 15 OR BETWEEN AUGUST 15 AND OCTOBER 1.
MULCH - UNROTTED STRAW OR HAY AT A RATE OF 70 TO 90 LBS./1,000 SF APPLIED TO ACHIEVE 95% SOIL SURFACE COVERAGE. MULCH SHALL BE ANCHORED BY APPROVED METHODS (I.E. PEG AND TWINE, MULCH NETTING, OR LIQUID MULCH BINDER).

I.B. PERMANENT SEEDING AND MULCHING:
TOPSOIL - UNIFORM APPLICATION TO A DEPTH OF 5" (UNSETTLED).
GROUND LIMESTONE - APPLIED UNIFORMLY ACCORDING TO SOIL TEST RECOMMENDATIONS.
FERTILIZER - APPLY 1 LBS./1,000 SF OF 10-10-10 OR EQUIVALENT WITH 50% WATER INSOLUBLE NITROGEN (UNLESS A SOIL TEST INDICATES OTHERWISE) WORKED INTO THE SOIL A MINIMUM OF 4".
SEED - TURF TYPE TALL FESCUE (BLEND OF 3 CULTIVARS) 350 LBS./ACRE (8 LBS./1,000 SF) OR OTHER APPROVED SEEDS; PLANT BETWEEN MARCH 1 AND OCTOBER 1 (SUMMER SEEDINGS REQUIRE IRRIGATION).
MULCH - UNROTTED STRAW OR HAY AT A RATE OF 70 TO 90 LBS./1,000 SF APPLIED TO ACHIEVE 95% SOIL SURFACE COVERAGE. MULCH SHALL BE ANCHORED BY APPROVED METHODS (I.E. PEG AND TWINE, MULCH NETTING, OR LIQUID MULCH BINDER).

DUST CONTROL NOTES

- MULCHES - SEE STANDARD OF STABILIZATION WITH MULCHES ONLY, PG. 5-1
- VEGETATIVE COVER - SEE STANDARD FOR TEMPORARY VEGETATIVE COVER, PG. 7-1
- PERMANENT VEGETATIVE COVER FOR SOIL STABILIZATION PG. 4-1 AND PERMANENT STABILIZATION WITH SOD, PG. 6-1
- SPRAY-ON ADHESIVES - ON MINERAL SOILS (NOT EFFECTIVE ON MUCK SOILS). KEEP TRAFFIC OFF THESE AREAS.
- TILLAGE - TO ROUGHEN SURFACE AND BRING CLODS TO THE SURFACE. THIS IS A TEMPORARY EMERGENCY MEASURE WHICH SHOULD BE USED BEFORE SOIL BLOWING STARTS. BEGIN PLOWING ON WINDWARD SIDE OF SITE. CHISEL-TYPE PLOWS SPACED ABOUT 12 INCHES APART AND SPRING-TOOTHED HARROWS ARE EXAMPLES OF EQUIPMENT WHICH MAY PRODUCE THE DESIRED EFFECT.
- SPRINKLING - SITE IS SPRINKLED UNTIL THE SURFACE IS WET.
- BARRIERS - SOLID BOARD FENCES, SNOW FENCES, BURLAP FENCES, CRATE WALLS, BALES OF HAY AND SIMILAR MATERIAL CAN BE USED TO CONTROL AIR CURRENTS AND SOIL BLOWING.
CALCIUM CHLORIDE - SHALL BE IN THE FORM OF LOOSE, DRY GRANULES OR FLAKES FINE ENOUGH TO FEED THROUGH COMMONLY USED SPREADERS AT A RATE THAT WILL KEEP SURFACE MOIST BUT NOT CAUSE POLLUTION OR PLANT DAMAGE. IF USED ON STEEPER SLOPES, THEN USE OTHER PRACTICES TO PREVENT WASHING INTO STREAMS OR ACCUMULATION AROUND PLANTS.
- STONE - COVER SURFACE WITH CRUSHED STONE OR COARSE GRAVEL.

NRCS WEB SOIL SURVEY SOIL CHARACTERISTICS CHART

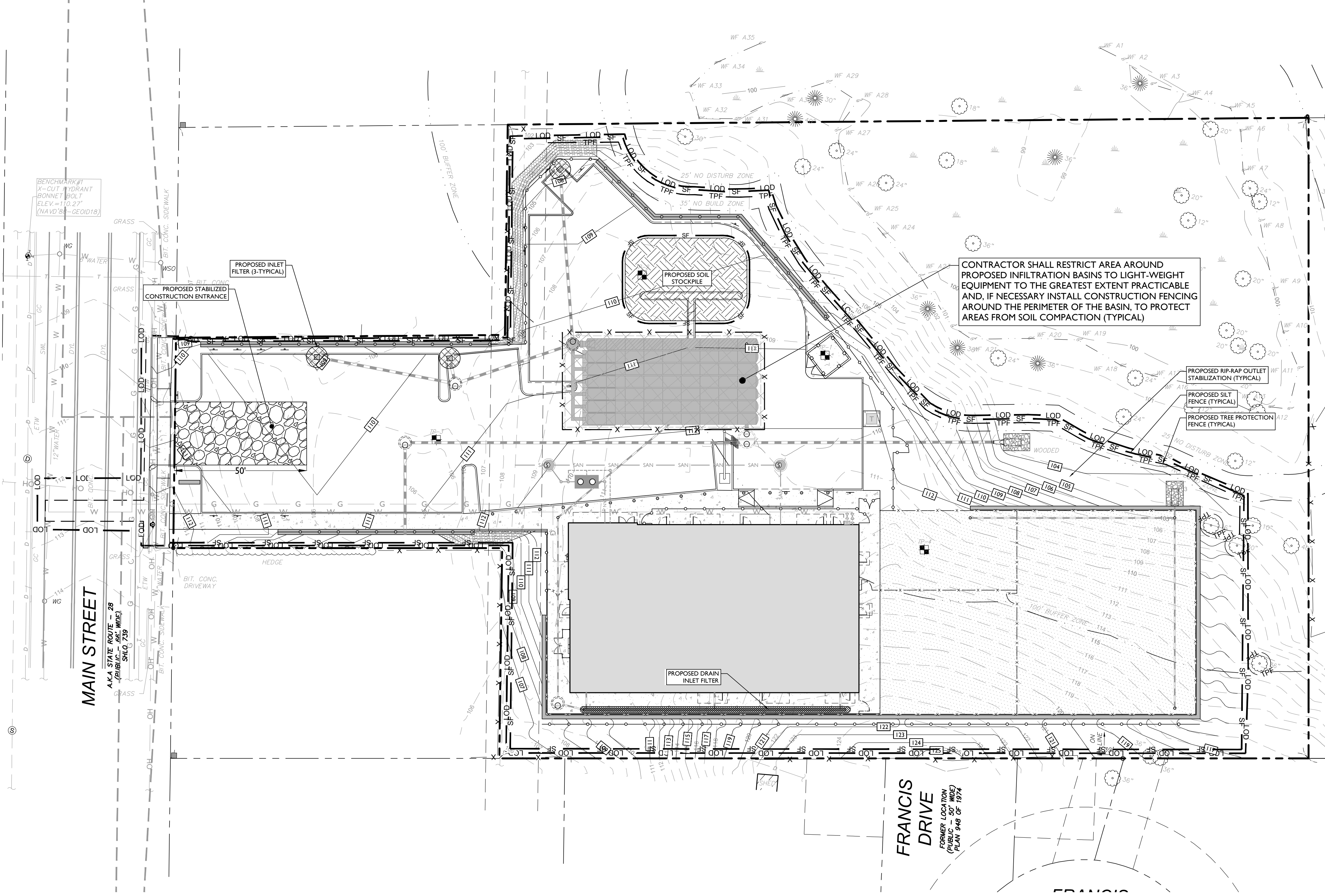
TYPE OF SOIL	WHITMAN FINE SANDY LOAM (73B)	CHARLTON-URBAN LAND-HOLLIS COMPLEX (631C)	UDORHENT'S (655)	PAXTON FINE SANDY LOAM (305C)	CANTON-CHARLTON-URBAN LAND COMPLEX (629C)
PERCENT OF SITE COVERAGE	61.3%	28.5%	5.9%	3.8%	0.5%
HYDROLOGIC SOIL GROUP	D	A	D ^s	C	A
DEPTH TO RESTRICTIVE LAYER	7 TO 38 INCHES	> 80 INCHES	> 80 INCHES	20 TO 39 INCHES	18 TO 30 INCHES
SOIL PERMEABILITY	0.00 TO 0.14 IN / HR	0.60 TO 6.00 IN / HR	*	0.00 TO 0.14 IN / HR	2.00 TO 6.00 IN / HR
DEPTH TO WATER TABLE	0 TO 6 INCHES	> 80 INCHES	> 80 INCHES	18 TO 37 INCHES	> 80 INCHES

* NOT SPECIFIED IN NRCS SOIL REPORT

- SYMBOL DESCRIPTION**
- PROPERTY BOUNDARY
 - ADJACENT PROPERTY BOUNDARY
 - LOD PROPOSED LIMIT OF DISTURBANCE
 - SF PROPOSED SILT FENCE
 - TPF PROPOSED TREE PROTECTION FENCE
 - PROPOSED STOCKPILE & EQUIPMENT STORAGE
 - PROPOSED STABILIZED CONSTRUCTION ENTRANCE
 - PROPOSED INLET PROTECTION FILTER

- SOIL EROSION AND SEDIMENT CONTROL NOTES**
- THE CONTRACTOR IS RESPONSIBLE FOR SOIL EROSION AND SEDIMENT CONTROL IN ACCORDANCE WITH LOCAL, STATE, AND FEDERAL REQUIREMENTS.
 - THE CONTRACTOR IS RESPONSIBLE FOR DUST CONTROL IN COMPLIANCE WITH LOCAL, STATE, AND FEDERAL AIR QUALITY STANDARDS.
 - THE CONTRACTOR IS RESPONSIBLE TO INSPECT ALL SOIL EROSION AND SEDIMENT CONTROL MEASURES WEEKLY AND AFTER A PRECIPITATION EVENT GREATER THAN 1 INCH. THE CONTRACTOR SHALL MAINTAIN AN INSPECTION LOG ON SITE AND DOCUMENT CORRECTIVE ACTION TAKEN THROUGHOUT THE COURSE OF CONSTRUCTION AS REQUIRED.

- SEQUENCE OF CONSTRUCTION**
- INSTALL CONSTRUCTION ENTRANCE (2 DAYS)
 - STRIPPING AND CLEARING OF SITE (2 WEEKS)
 - INSTALL CURBSIDE SEDIMENT BARRIERS (1 DAY)
 - DEMOLISH EXISTING PAVEMENT WHERE APPLICABLE (7 DAYS)
 - ROUGH GRADING AND TEMPORARY SEEDING (21 DAYS)
 - BASE CONSTRUCTION INCLUDING STABILIZATION (14 DAYS)
 - UTILITY CONSTRUCTION (10 DAYS)
 - BUILDING CONSTRUCTION AND SITE IMPROVEMENTS (100 DAYS)
 - FINAL GRADING (3 DAYS)
 - SOIL RESTORATION MEASURES (3 DAYS)
 - LANDSCAPING IMPROVEMENTS AND FINAL SEEDING & TOP SOILING (7 DAYS)
 - REMOVE SOIL EROSION MEASURES (1 DAY)
- NOTE: TIME DURATIONS ARE APPROXIMATE AND ARE INTENDED TO ACT AS A GENERAL GUIDE TO THE CONSTRUCTION TIMELINE. ALL DURATIONS ARE SUBJECT TO CHANGE BY CONTRACTOR. CONTRACTOR SHALL SUBMIT CONSTRUCTION SCHEDULE TO TOWNSHIP AND ENGINEER. CONTRACTOR SHALL PHASE CONSTRUCTION ACCORDINGLY.



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SCALE: 1" = 20' PROJECT ID: BOS-240115

TITLE: **SOIL EROSION & SEDIMENT CONTROL PLAN**

DRAWING: **C-9**

811

Know what's below
Call before you dig.

20' 0' 20' 40'
GRAPHIC SCALE IN FEET
1" = 20'

Z:\BOSTON\2024\240115 PRIMROSE SCHOOLS - 885 MAIN STREET, MIDDIX, MA\CAD\DWG\1504\1504.DWG

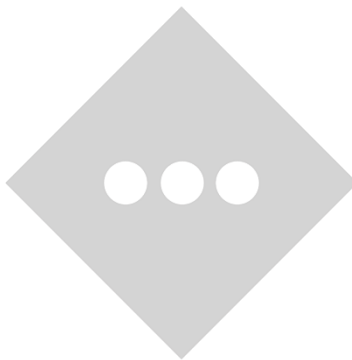
APPENDIX E

DRAINAGE AREA MAPS

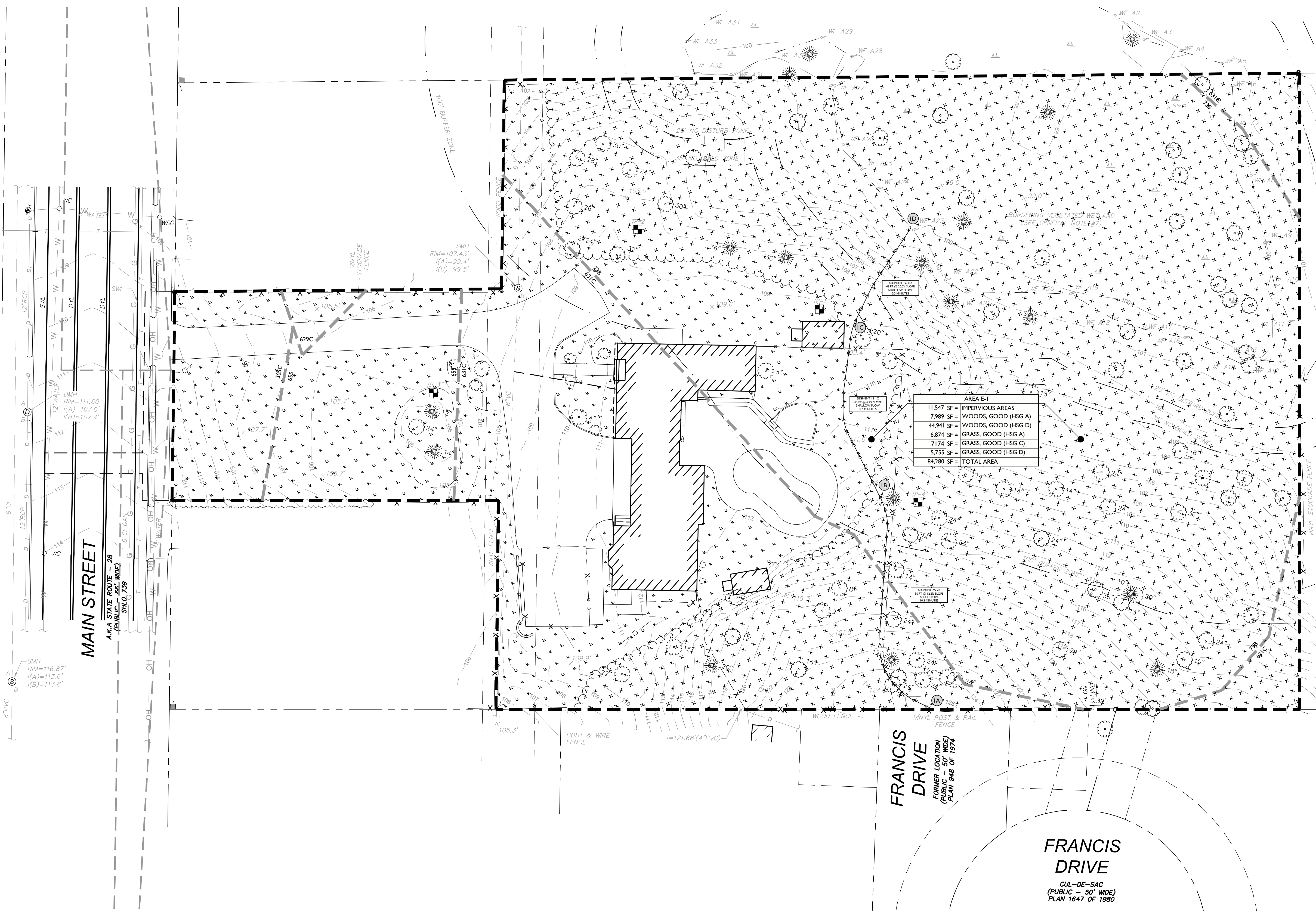
INVENTORY

SHEET 1 OF 2: EXISTING DRAINAGE AREA MAP

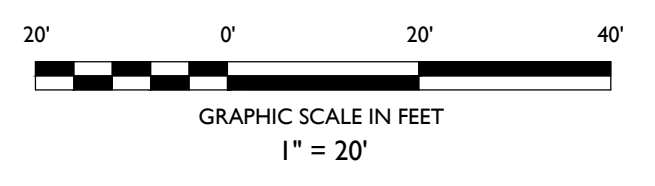
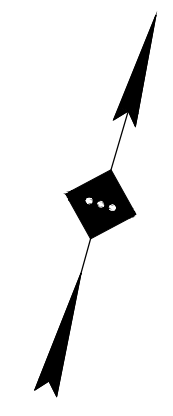
SHEET 2 OF 2: PROPOSED DRAINAGE AREA MAP



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SYMBOL	DESCRIPTION
	PROPERTY LINE
	ADJACENT PROPERTY LINE
	EXISTING DRAINAGE AREA
	TIME OF CONCENTRATION PATH
	EXISTING GRASS AREA
	EXISTING WOODED AREA



ISSUE	DATE	BY	DESCRIPTION
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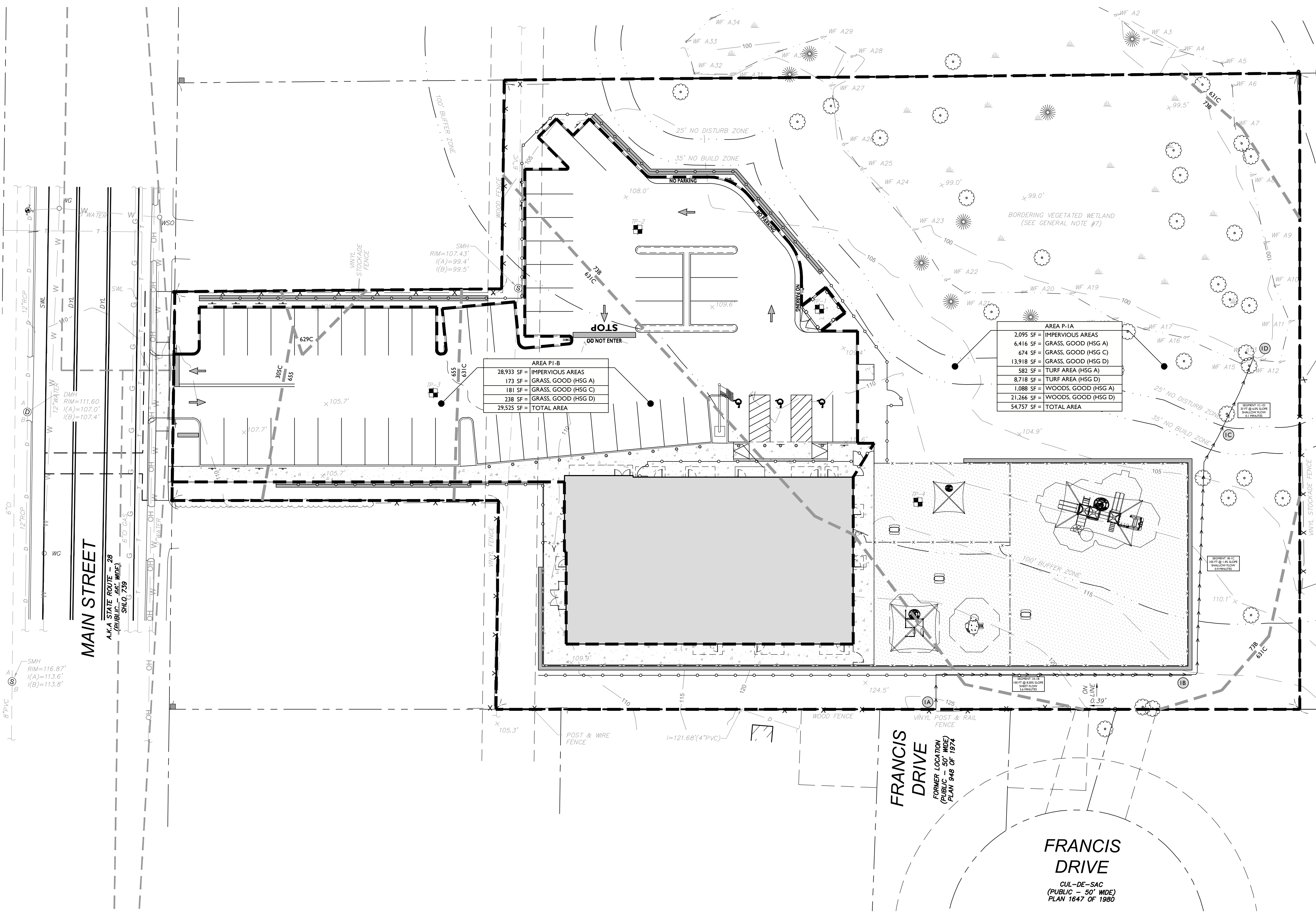
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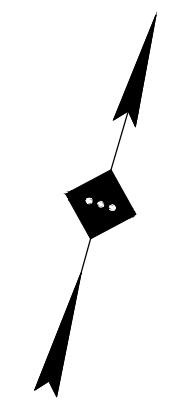
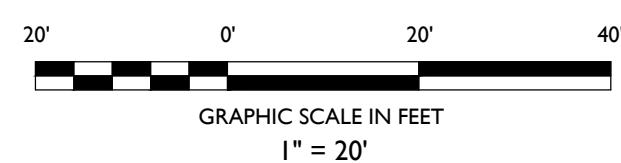
TITLE:
**EXISTING DRAINAGE
AREA MAP**

DRAWING:
1 OF 2

Z:\PROJECTS\2025\240115 PRIMROSE SCHOOLS - 885 MAIN STREET, MIDDLETOWN, MA\CAD\DWG\DRAINAGE AREA MAP.DWG



SYMBOL	DESCRIPTION
---	PROPERTY LINE
- - -	ADJACENT PROPERTY LINE
- . - . -	PROPOSED DRAINAGE AREA
→ →	TIME OF CONCENTRATION PATH



ISSUE	DATE	BY	DESCRIPTION
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SCALE: 1" = 20' PROJECT ID: BOS-240115

TITLE:
**PROPOSED DRAINAGE
AREA MAP**

DRAWING:

2 OF 2